



CADMAN MATERIALS INC.  
Canby Pit – Phase 4 Expansion

March 4, 2019

# Traffic Analysis

## Cadman Materials



RENEWAL 06/30/20

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Canby, Oregon  
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## EXECUTIVE SUMMARY

Cadman Materials Inc. (Cadman) is requesting a comprehensive plan amendment and related application of the MAO overlay zone designation to properties west of South Barlow Road in Canby, Oregon. The requested plan amendment and zone change are to allow for the future extraction of aggregate. This report summarizes the traffic impact analysis necessary to satisfy the requirements of Oregon Administrative Rule (OAR) 660-12-0060 (the Transportation Planning Rule or “TPR”) for the proposed zone change and comprehensive plan amendment and OAR 660-23-180(4) (b) (B) (the Goal 5 implementing rule or the “Goal 5 Rule”) for the mining operation.

## FINDINGS

This analysis includes the following findings:

- Highway 99E at South Barlow Road currently does not meet the adopted mobility standard which is expected to continue through Clackamas County’s Transportation System Plan planning horizon. Oregon Highway Plan Action 1F.5 defines the mobility standard as no further degradation for this intersection. Since the application requests authorization of a minable inventory of aggregate resource to replace depleted resources at the facility, traffic associated with the application of the MAO overlay from the proposed aggregate extraction operation does not modify the volume-to-capacity ratios beyond existing background conditions, meeting ODOT mobility targets as defined in the Oregon Highway Plan.
- The existing site driveway will meet the mobility standard for the year of opening and through the end of Clackamas County Transportation System Plan planning horizon with the comprehensive plan amendment.
- The existing horizontal alignment of all roadways can accommodate truck traffic consistent with applicable standards under Goal 5.
- The application was found to meet all applicable traffic analysis code criteria.
- TPR has been demonstrated to be met for the proposed zone change and comprehensive plan amendment.

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## 1.0 BACKGROUND

Cadman Materials Inc. (Cadman) is proposing the inclusion of 99 acres adjacent to their existing aggregate operation southwest of Canby, Oregon. The site is located on the west side of South Barlow Road south of Highway 99E. The applicant is requesting a comprehensive plan amendment to include the site on the County's inventory of significant aggregate resource sites and the application of the Mineral Aggregate Overlay (MAO) to support the proposed aggregate extraction. The inclusion of the 99 acres, which is currently zoned Exclusive Farm Use (EFU) into the existing operations will allow Cadman to continue its current extraction and processing activities at this location by providing a source of new aggregate as the resources on the existing site are depleted. Cadman does not propose to increase production of aggregate over the currently allowed 3 million tons/year, or the associated number of truck trips. Additionally, the existing conveyor tunnel currently used to transfer aggregate from the west side to the east side of South Barlow Road will continue to operate as Cadman moves into this area which eliminates the need for any additional truck traffic.

### 1.1 PROPOSED SITE INFORMATION

Cadman is proposing to incorporate eight adjacent tax lots to their existing aggregate facility along South Barlow Road south of Highway 99. The existing facility is comprised of the following tax lots:

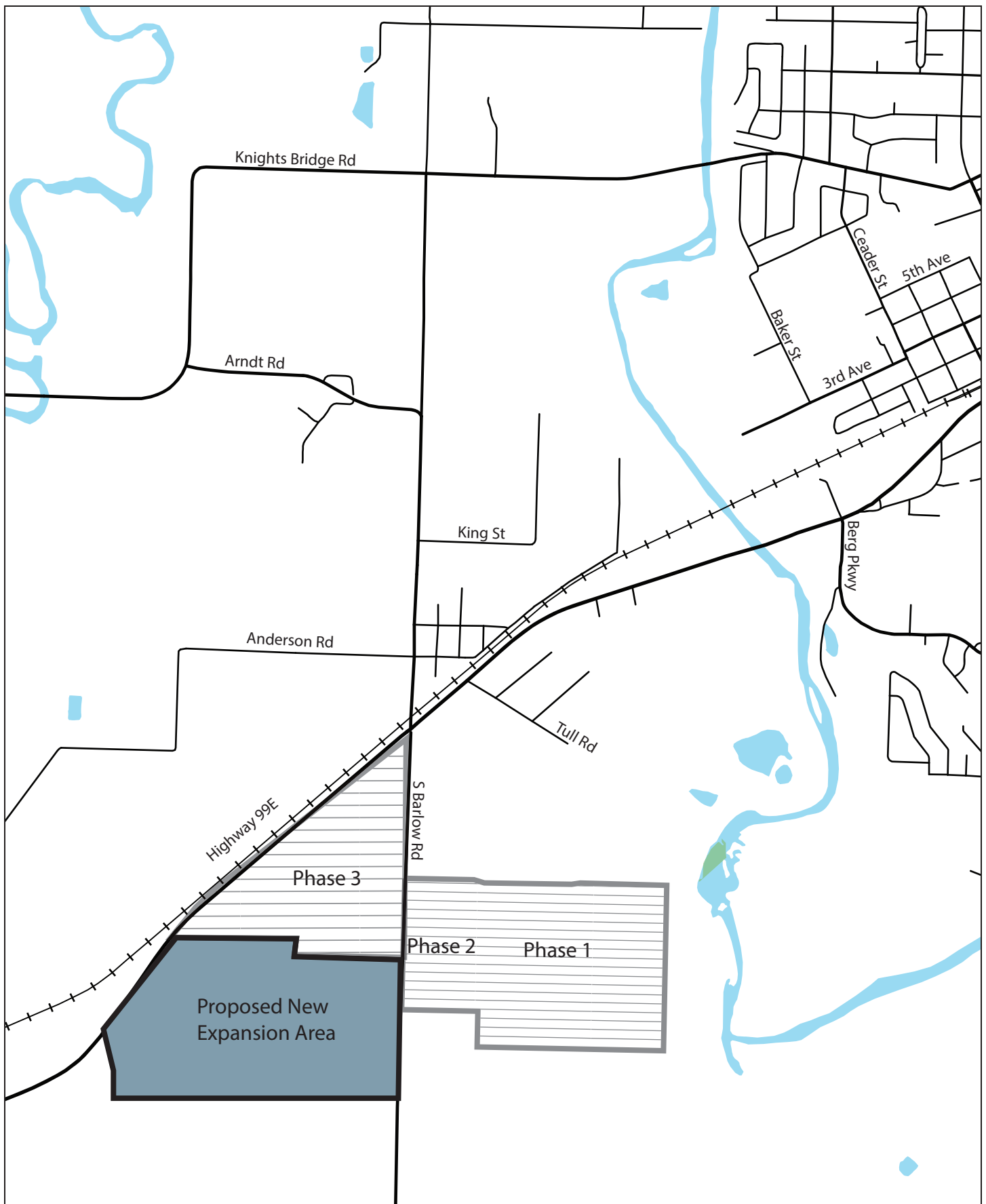
- 41E06- 1900
- 41E07- 100, 300, 190, 400, 390
- 41E08- 1000, 800, 700

The proposal is to add the tax lots illustrated in Table 1 within the existing operations. Figure 1 provides a vicinity map of the site. Appendix A contains more detail of the tax lots.

TABLE 1: PROPOSED TAX LOTS

<b>Tax Lot</b>	<b>Acres</b>	<b>Zoning</b>
41E07- 1003	17.0	EFU
41E07- 1004	17.2	EFU
41E07- 1002	7.3	EFU
41E07- 500	11.3	EFU
41E07- 600	27.7	EFU
41E07- 800	14.38	EFU
41E07- 801	1.4	EFU
41E07- 700	3.93	EFU

The proposed tax lots are currently zoned Exclusive Farm Use (EFU).



Cadman, Canby, Oregon

Figure 1: Site Location and Vicinity Map

## 1.2 OPERATIONAL BACKGROUND APPROVALS AND PHASING

**Phase 1:** The original aggregate operation was approved in 1999 and was contained within the tax lots located on the east side of South Barlow Road. In 2000, the site was approved for an increase in production to 3 million tons/year. With the approval, Clackamas County placed a trip cap on the property of no more than 154 AM peak hour and 120 PM peak hour trips.

**Phase 2:** In 2007, the operation incorporated tax lot 1000 on the east side of South Barlow Road, known as the “Rodrigues” property. The incorporation of this tax lot allowed the current/approved operations to be maintained within the 3 million tons/year limit. As the sand and gravel were depleted in the existing site, operations at the “Rodrigues” site were added to maintain production. All processing, internal roadways, and site access remained as originally permitted in Phase 1. There was no increase in production or increase in vehicle trips to the site from the inclusion of this tax lot.

A traffic impact evaluation was prepared for this phase. The evaluation determined that the Transportation Planning Rule (TPR) was met.

**Phase 3:** In 2012, Phase 3 was approved by the County to extend the MAO overlay designation to several tax lots on the west side of South Barlow Road. The inclusion of these lots in the MAO overlay allowed the continued operation at the existing location, subject to the permitted capacity of up to 3 million tons/year and continuation of the trip cap of 154 AM and 120 PM peak hour trips.

With this phase, a conveyor tunnel was built under South Barlow Road. All aggregate material extracted from the tax lots on the west side is transferred to the east side via the conveyor tunnel. All truck traffic uses the existing phase 1 and phase 2 driveway onto the east side of South Barlow Road.

## 1.3 SITE OPERATION

As the existing lands designated MAO are depleted of aggregate, Cadman is proposing the application of the MAO overlay designation to additional tax lots to allow mining within this additional area upon depletion of the current MAO area. Approval of this area for mining will allow the existing operation to be maintained in its current location.

The existing aggregate facility is allowed to produce up to 3 million tons/year, subject to a trip cap of 154 AM peak hours trips and 120 PM peak hour trips; this will not change with the proposed expansion. There will be no additional vehicle trips beyond what is already generated from the existing operations at Phases 1 - 3.

Additionally, the conveyor tunnel will continue to be used for the same purpose. All material will be transferred from the west side to the east side. Access to the tax lots on the west side of

South Barlow Road will be limited to the occasional equipment and employee trips with all trucks utilizing only the existing access to tax lot 1000.

#### 1.4 EVALUATION CRITERIA

The applicant is requesting a comprehensive plan amendment to (1) include the Site on the Significant Inventory and (2) add a Mineral Aggregate Overlay (MAO) designation to support the proposed aggregate extraction operation. Impacts to the surrounding transportation system from the proposed zone change and mining operation are analyzed and evaluated under the following criteria:

- Oregon Administrative Ruling (OAR 660-23-180 (5)(b)(B)), Goal 5, for the mining operation
- OAR 660-12-0060, Goal 12, Transportation Planning Rule (TPR) for the Comprehensive Plan Amendment

**Goal 5 (specifically, OAR 660-23-180(5)(b))** requires that local governments determine existing and approved land uses within the impact area that will be adversely affected by proposed mining operations and specify the predicted conflicts. For determination of traffic conflicts from proposed mining of a significant aggregate site, the local government shall limit its consideration to the following:

“(B) Potential conflicts to local roads used for access and egress to the mining site within one mile of the entrance of the mining site unless a greater distance is necessary in order to include the intersection with the nearest arterial identified in the local transportation plan. Conflicts shall be determined based on clear and objective standards regarding sight distances, road capacity, cross-section elements, horizontal and vertical alignment, and similar items in the transportation plan and implementing ordinances. Such standards for trucks associated with the mining operation shall be equivalent to standards for other trucks of equivalent size, weight, and capacity that haul other materials;” OAR 660-23-180(5)(b)(B).

**Goal 12 (specifically OAR 660-12-0060 (1))** requires that a local government ensures that an amendment to a functional plan, an acknowledged comprehensive plan, or a land use regulation (including a zoning map) does not significantly affect an existing or planned transportation facility. A plan or land use amendment significantly affects a transportation facility if it would:

- “(a) Change the functional classification of an existing or planned transportation facility (exclusive of correction of map errors in an adopted plan);
  - (b) Change standards implementing a functional classification system; or
  - (c) Result in any of the effects listed in paragraphs (A) through (C) of this subsection based on projected conditions measured at the end of the planning period identified in the adopted TSP. As part of evaluating projected conditions, the amount of traffic projected to be generated within the area of the amendment may be reduced if the amendment includes an enforceable, ongoing requirement that would demonstrably limit traffic generation, including, but not limited to, transportation demand management. This reduction may diminish or completely eliminate the significant effect of the amendment.
- (A) Types or levels of travel or access that are inconsistent with the functional classification of an existing or planned transportation facility;
  - (B) Degrade the performance of an existing or planned transportation facility such that it would not meet the performance standards identified in the TSP or comprehensive plan; or
  - (C) Degrade the performance of an existing or planned transportation facility that is otherwise projected to not meet the performance standards identified in the TSP or comprehensive plan.” OAR 660-12-0060(1)

## 1.5 SCOPE OF ANALYSIS

The applicant is planning to maintain the existing access connection and usage on South Barlow Road. South Barlow Road is classified as a major arterial within Clackamas County Transportation System Plan<sup>1</sup>.

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<sup>1</sup> Scope of work is based on phone conversations with ODOT and Clackamas County. The evaluation is consistent with our understanding that TPR was the main issue to address for this application.

**Goal 5 Criteria:**

Goal 5 requires an analysis of potential transportation impacts to **local roads** used for access and egress to the mine site within one mile of the entrance or to the nearest arterial if a greater distance is necessary. Vehicle traffic generated by the subject Tax Lots will have direct access to South Barlow Road which is considered a “major arterial” by the Clackamas County Transportation System Plan.

The nearest major intersection is Highway 99E at South Barlow Road. This is the only intersection within one (1) mile that will receive regular turning truck traffic; all other intersections have minimal additional through traveling trucks.

This transportation analysis evaluates conflicts from truck traffic generated by the site from the driveway to the intersection of Highway 99E and South Barlow Road. This analysis includes the following:

- Estimation of the number of trucks expected to be generated by the site during the peak traffic demand period and over a whole day for normal operating conditions.
- Evaluation of the Site Driveway for truck access on South Barlow Road for existing conditions and the planning horizon of the Clackamas County TSP.
- Evaluation of intersection operations for the existing year (Year 2019), and at the end of the planning horizon as per Clackamas County’s Transportation System Plan (Year 2033). The analysis compares conditions with and without additional trips, if any, associated with the proposed aggregate extraction operations and compares the intersection operations to ODOT’s mobility standards, for the PM peak hour.
- Evaluation of roadway geometry/cross-sectional elements along South Barlow Road from the site driveway to the intersection of Highway 99E and South Barlow Road to determine if there are any horizontal or vertical alignment issues.
- Evaluation of sight distance at the site entrance.
- Evaluation of crash data at the site entrance and the intersection of Highway 99E and South Barlow Road.

**Transportation Planning Rule (TPR) Criteria:**

Compliance with TPR analysis requirements includes:

- Determining the number of trips to the site under the existing zoning and proposed zoning using a “reasonable worst-case” development potential.

## 2.0 PROPOSED SITE USAGE AND OPERATIONS

### 2.1 TRUCK USAGE

Cadman's aggregate operation at the Canby Pit has been permitted in prior approval of adjacent operations to allow no more than 3 million tons/year and no more than 154 AM peak hour trips and 120 PM peak hour trips. The proposed expansion to include the additional tax lots will allow Cadman to continue the existing operations at this location upon depletion of minable resources at the existing site, without an increase to production over what is currently allowed.

### 2.2 ACCESS

All aggregate from this site will be conveyed to the existing processing facility located on the east side of South Barlow Road for processing. All trucks entering and leaving the overall site will access the operations using the existing access on the east side of South Barlow Road. No vehicular traffic will use the access on the west side of South Barlow Road.

Figure 2 provides the site configuration.

## 3.0 EXISTING CONDITIONS

### 3.1 STREET NETWORK

#### **Highway 99E**

Highway 99E, is under the jurisdiction of the ODOT and is classified a Regional Highway. Highway 99E is a five-lane highway with an average width of 90 feet. Lane widths along Highway 99E are 12 feet with a 17-foot two-way left-turn lane (TWLTL) and 10 to 11-foot shoulders. The posted speed is 55 MPH.

#### **South Barlow Road**

South Barlow Road is classified as a Major Arterial. It is a two-lane, two-way road. South Barlow Road has an average width of 24 feet. Lane widths in this segment average 12 feet with gravel shoulders. The road is striped with a centerline and fog lines. South Barlow Road enters the Barlow Urban Growth Boundary north of Highway 99E and otherwise is outside of the Urban Growth Boundary. South Barlow Road is under Clackamas County Jurisdiction and has a posted speed of 55 MPH.

### 3.2 INTERSECTION CONFIGURATION

The signalized intersection of Highway 99E at South Barlow Road is evaluated for performance. Figure 4 shows the existing lane configurations and traffic control for the studied area.



Cadman, Canby, Oregon

Figure 2: Site Access

### 3.3 CRASH ANALYSIS

Pursuant to OAR 660-023-160 (5)(B) crash investigation was performed for the study area intersection and site access. The analysis investigates crashes that have been reported to the state for the most recent 5 years, January 1, 2013 to December 31, 2017, following procedures set forth in ODOT's Analysis Procedures Manual (APM). The study area is not large enough to use the HCM Critical Rate methodology for intersections or roadway segments. Therefore, the intersection and segment crash rate were used. The segment crash rate determines a crash rate in crashes per million vehicle miles of travel on the roadway segment. The crash rate is compared to the statewide average crash rate for roads of the same functional classifications. If the calculated crash rate exceeds the crash rate for that functional classification or there is a high percentage of a certain crash type, the location is investigated for further mitigation measures. Crash data was provided by ODOT and is included in Appendix B. The results of the segment crash analysis are provided in Table 2.

TABLE 2: SEGMENT CRASH RATE – SOUTH BARLOW ROAD (1.0 MILES SOUTH OF SITE DRIVEWAY TO HIGHWAY 99)

Collision Type	Year					Total
	2013	2014	2015	2016	2017	
Angle	0	0	0	0	0	0
Fixed/Other Object	0	0	0	0	0	0
Head-On	0	0	0	0	0	0
Rear-End	0	0	0	0	0	0
Sideswipe-Overtaking	0	0	1	1	1	3
Turning Movement	0	0	0	0	0	0
Other	1	0	1	0	0	2
<b>Total</b>	<b>1</b>	<b>0</b>	<b>2</b>	<b>1</b>	<b>1</b>	<b>5</b>
ADT	5268	5417	5560	5716	5865	
<b>Crash Rate*</b>	<b>0.27</b>	<b>0.00</b>	<b>0.51</b>	<b>0.25</b>	<b>0.24</b>	<b>0.25</b>
<b>Statewide Crash Rate</b>	2013	204	2015	2016	2017	
Rural Area – Other Minor Arterials	1.15	1.22	1.24	1.35	**	1.14

\*crashes/million vehicle miles (normalized over one mile)

\*\* Rates not provided by ODOT as of the date of the report

As illustrated in Table 2, the studied segment of South Barlow Road has an average crash rate of 0.25 which is lower than the average State Highway Crash Rate, 1.14, for minor arterials in rural areas over the last 5 reported years.

Additionally, an intersection crash analysis was completed for the intersection of Highway 99E and South Barlow Road considering only crashes which occurred within the intersection influence area. The intersection crash rate is evaluated in terms of crashes per million entering

vehicles at the intersection. The crash rate is compared to the statewide 90<sup>th</sup> percentile intersection crash rate in Exhibit 4-1 in the APM. The result of the intersection crash analysis is provided in Table 3.

TABLE 3: INTERSECTION CRASH RATE-SOUTH BARLOW AT HIGHWAY 99

Location	Number of Crashes	Types of Crashes						ADT	Crash Rate*	Statewide 90 <sup>th</sup> Percentile Crash Rate
		Head	Rear	Side	Turn	Other	Pedestrian/Bike			
Highway 99E at S Barlow Rd	41	0	14	5	16	6	0	40,000	0.56	0.579
S Barlow Road at Site Driveway	0	0	0	0	0	0	0	5865	0.00	0.475

\*(crashes/million entering vehicles)

As illustrated in Table 3, the intersection of Highway 99E and South Barlow Road has a crash rate of 0.56 which is lower than the statewide 90<sup>th</sup> percentile intersection crash rate, 0.579, for rural 4-leg signalized intersections. The Site Driveway at South Barlow Road has a crash rate of 0.00 which is lower than the 90<sup>th</sup> percentile intersection crash rate, 0.475, for rural 3-leg minor stop-controlled intersections.

### 3.4 EXISTING TRAFFIC VOLUMES

#### INTERSECTION COUNTS

As part of the analysis, PM peak hour turning movement counts were collected at the intersection of Highway 99E and South Barlow Road. The traffic count was performed for the weekday peak periods of 3:30 PM to 6:00 PM.

The turning movement counts illustrate that the weekday PM peak hour occurs from 4:15-5:15 PM. The traffic volumes are included in Appendix C.

#### SEASONAL ADJUSTMENT

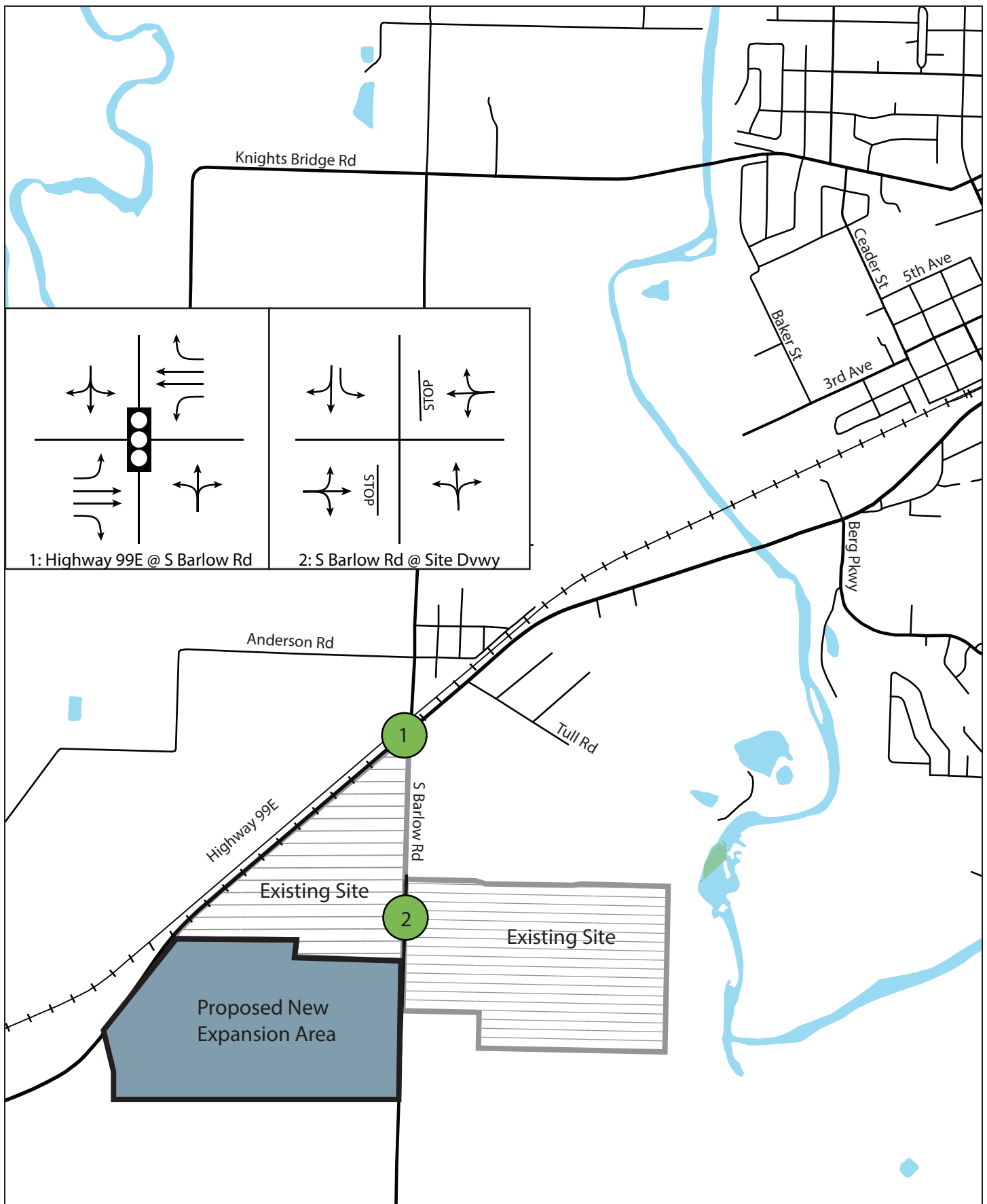
Application of seasonal adjustment factors account for the fact that through volumes along State Highways and recreational routes tend to fluctuate from month to month due to changes in recreational behavior, etc. Monthly volume variations for routes with recreational traffic show much higher seasonal peaking than for traffic with predominantly intercity traffic. The design hour traffic volumes are adjusted to reflect traffic conditions on roadways during the peak month of the year using the seasonal adjustment factor.

There are no Automatic Traffic Recorders (ATR) in the vicinity located on Highway 99E. Therefore, following methodology outlined by ODOT's *Analysis Procedures Manual (APM)*, the

Seasonal Trend Table was used to seasonally adjust traffic counts taken in September to the peak month of the year, August. Specifically, the commuter trend was determined to be most appropriate. The seasonal adjustment factor was determined to be 1.026. The seasonal adjustment calculation is included in Appendix C.

#### DESIGN HOUR VOLUMES

The existing traffic volumes were adjusted according to the methodology described above. Appendix C provides the traffic volume calculations.



Cadman, Canby, Oregon

Figure 3: Lane Configuration and Intersection Control

## 4.0 INTERSECTION EVALUATION

Goal 5 requires an evaluation of traffic operations at intersections that will be impacted by the proposed mining operation.

The intersections of Highway 99E at South Barlow Road and the Site Access at South Barlow Road were evaluated to determine if the proposed zone change and development will significantly affect these intersections as defined in Goal 5.

### 4.1 PERFORMANCE MEASURES

Intersections within this study were evaluated for volume-to-capacity ratios (V/C) and Level of Service (LOS). The volume-to-capacity ratio describes the capability of an intersection to meet volume demand based upon the maximum number of vehicles that could be served in an hour. Level of Service quantifies the degree of comfort a driver experiences as they travel through an intersection or roadway, measured in a delay in second per vehicle.

V/C is the threshold for which ODOT evaluates the operation of intersections, as defined by the *1999 Oregon Highway Plan*. V/C thresholds are defined based on roadway classification and speed. Since Highway 99E is a Regional Highway, the maximum v/c threshold for this facility type is 0.75.

Level of Service is the performance measure for rural roadway segments and unsignalized intersections within Clackamas County. LOS E is the standard for these facility types.

For this study, volume-to capacity intersection analysis was completed according to the *Highway Capacity Manual* (HCM) method implemented in SYNCHRO Version 9.

### 4.2 BASE TRAFFIC VOLUMES

The existing year base traffic volumes were collected and seasonally adjusted as per the methodology described in Section 3.4.

### 4.3 FUTURE YEAR BACKGROUND VOLUMES

Consistent with the traffic impact analysis criteria the intersections were evaluated for the existing condition, year 2019, and the planning horizon as per the Clackamas County Transportation System Plan (TSP), year 2033. The growth rate was calculated using existing and future condition traffic volumes from the Clackamas County TSP. The growth rate was calculated by approach with rates ranging from 1% for Highway 99 and 2.5% for South Barlow Road.

The growth rate calculations are included in Appendix C. Figure 4 illustrates the year 2019 and 2033 background traffic volumes.

#### 4.5 SITE GENERATED TRIPS

Cadman has been permitted to allow up to but not more than 120 PM peak hour trips associated with its operations at this location. The evaluation considered operations up to the trip cap.

#### 4.6 SITE TRIP DISTRIBUTION AND ASSIGNMENT

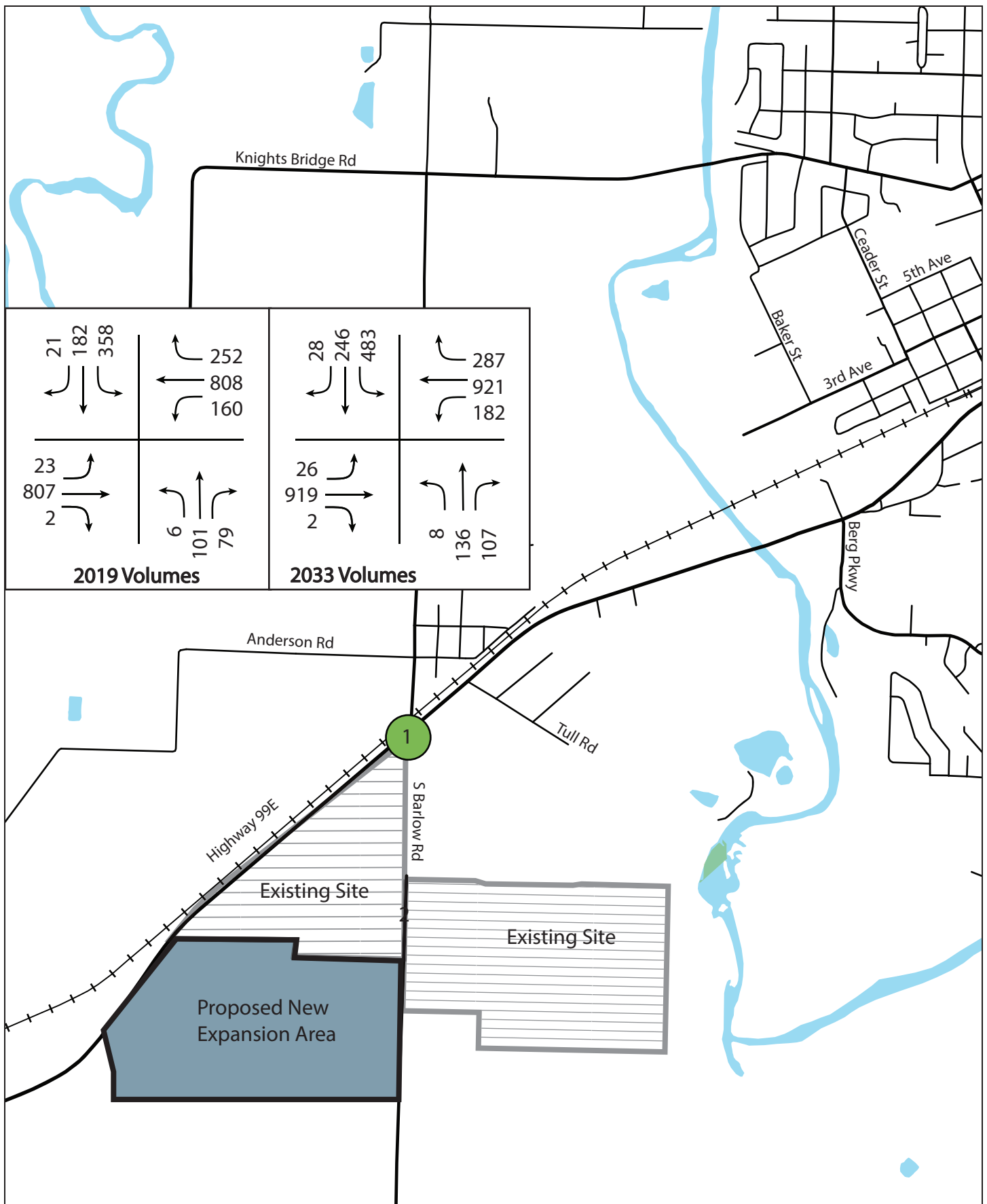
The truck trips are assumed to follow the following splits:

- 40% to/from the east on Highway 99E
- 45% to/from the north on South Barlow Road
- 5% to/from the west on Highway 99E
- 10% to/from the south on South Barlow Road

The PM peak hour trips were distributed along the haul route as illustrated in Figure 5.

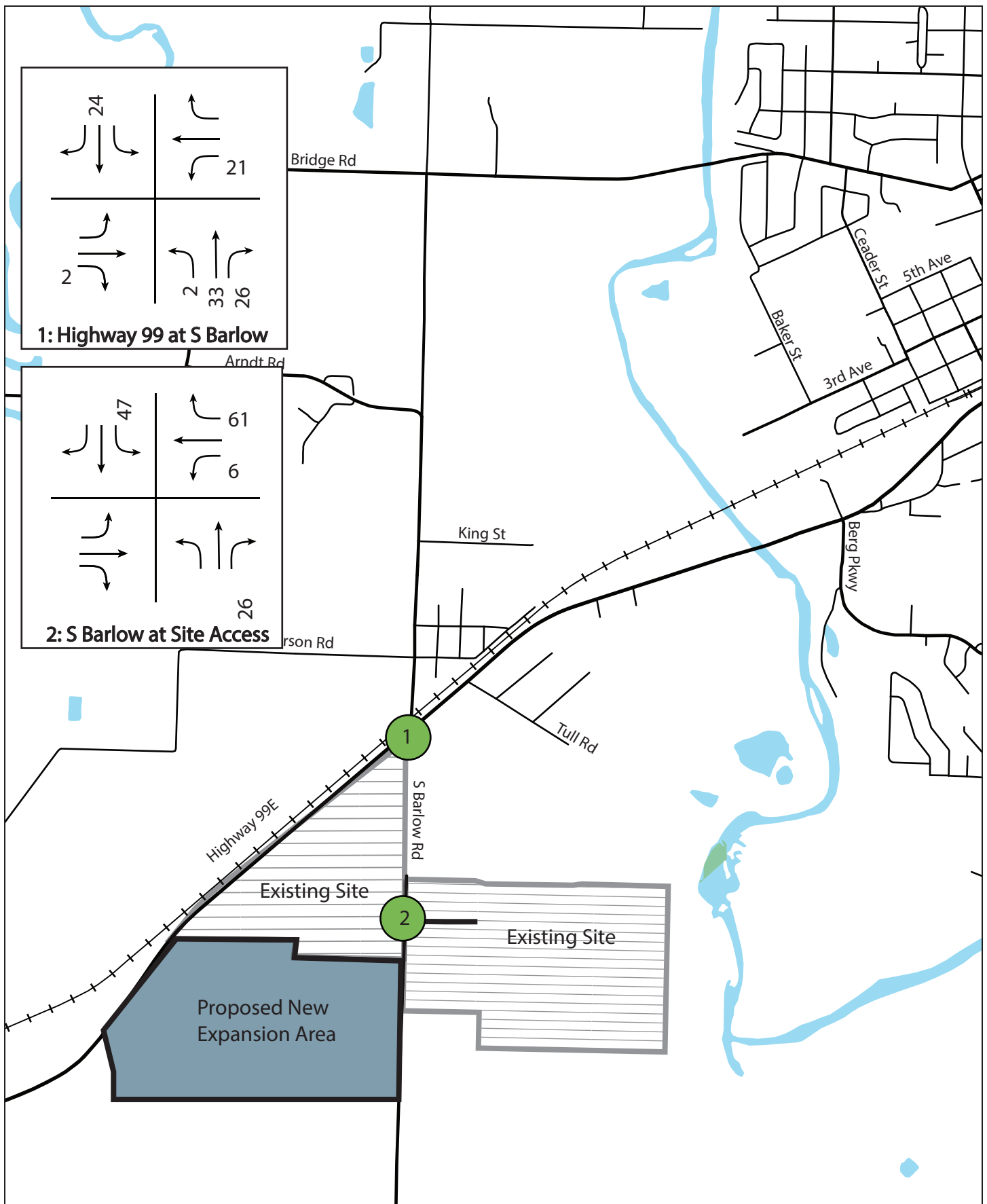
#### 4.7 INTERSECTION VOLUMES WITH SITE TRIPS

The proposed site trips were added to the year 2033 base traffic volumes to represent volumes with the site traffic added to it. Figures 6 illustrates traffic volumes for the year 2033 within the site operation.



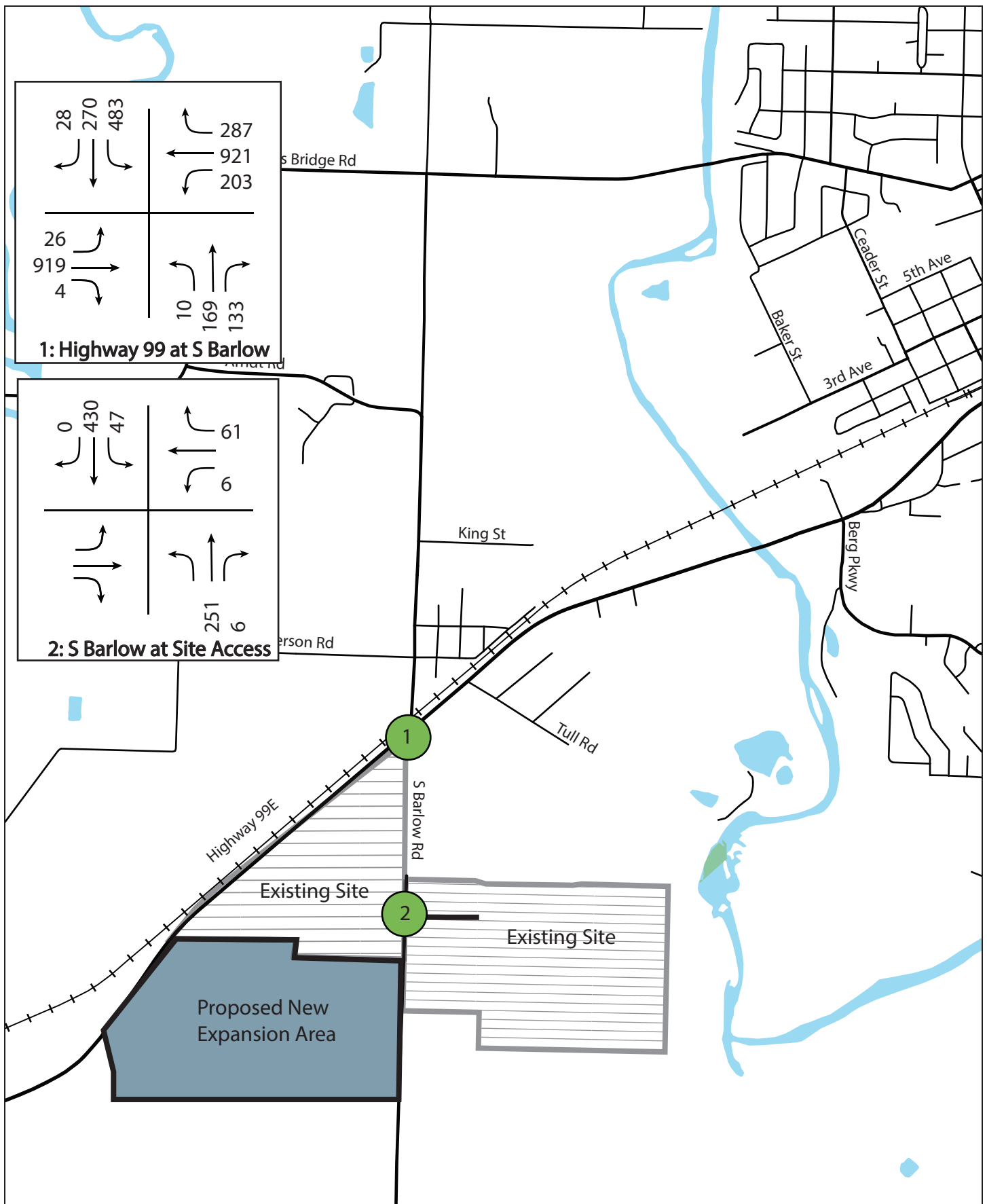
Cadman, Canby, Oregon

Figure 4: PM Peak Hour Background Traffic Volumes



Cadman, Canby, Oregon

Figure 5: PM Peak Hour Site Trips



Cadman, Canby, Oregon

Figure 6: 2033 PM Peak Hour with Site Operation

#### 4.8 INTERSECTION ANALYSIS RESULTS

A performance analysis was conducted for the studied intersections for the year 2019 existing conditions for the PM peak hour, and year 2033 conditions with and without the site operations. The analysis included mitigations provided on Clackamas County's TSP list of financially constrained projects. Specifically, at the intersection of Highway 99E at South Barlow Road, the financially constrained project list includes the construction of two southbound left turn lanes which were considered in this analysis. The results of the analysis are presented in Table 4. The SYNCHRO outputs are provided in Appendix D.

TABLE 4: INTERSECTION PERFORMANCE: YEAR WEEKDAY PM PEAK HOUR

Intersection	Mobility Standard	2019 Existing	2033 No-Build*	2033 Build*
Highway 99E at S Barlow Rd	V/C 0.75	1.00	0.89	0.89
South Barlow Road @ at Site Driveway	LOS E	N/A	B	B

\*Includes the intersection improvements

As shown above, the intersection of Highway 99E at South Barlow Road exceeds the accepted mobility standard in the existing year and 2033 background condition. The continuation of the Cadman operations does not worsen the operation of the intersection. The site driveway continues to operate within the mobility standard under the full trip cap of 120 PM peak hour trips.

The Oregon Highway Plan Action 1F.5 states that for the purpose of evaluating amendments to a comprehensive plan, in situations where the volume to capacity ratio is currently above the mobility targets or is projected to be above the mobility targets at the planning horizon, the mobility target is to avoid further degradation. As shown, in Table 4, the proposed aggregate expansion (Canby Phase 4) and associated comprehensive plan amendment does not worsen the v/c ratio beyond the background conditions which meets the criteria of the Oregon Highway Plan, thus meeting the applicable mobility standards.

As shown in Table 4, the v/c is projected to be 0.89 in the year 2033 for Highway 99E at South Barlow Road. This is better than the 1.00 in the current year 2019. As stated above, Clackamas County's capital improvement list has identified this intersection for improvements. The identified improvement of dual southbound left turn lanes will improve the v/c to 0.89 by the end of the planning horizon.

## 5.0 GOAL 5 EVALUATION

As per Goal 5, local roads within one (1) mile of the site entrance that are used for access and egress from the site are to be evaluated for road capacity, cross section elements, and horizontal and vertical alignment. The site access is on South Barlow Road which is a major arterial. Regular truck traffic will be to the north to Highway 99E, a state facility. To the south on South Barlow Road, the 1 mile is reached just before the intersection of South Lone Elder Road, a minor arterial.

It is assumed that trucks accessing the site will be a mix of single unit dump trucks and dump trucks with a pup trailer. The evaluation takes into consideration both types of trucks.

### 5.1 SIGHT DISTANCE

Sight distances are classified by the stopping sight distance (SSD) for the major roadway and departure/intersection sight distance (ISD) for the minor street (controlled) approach. The stopping sight distance is the length of roadway needed for a vehicle traveling at the design speed to safely stop for a stationary object in the roadway. The required sight distance allows a driver to perceive and react to an object two (2) feet high on the roadway visible from a driver's eye height of 3.5 feet above the ground. The departure sight distance (ISD) is a measure of length of visibility of the roadway given to a stopped driver on a minor road approach. The distance provides time to perceive and react to gaps in traffic. For this calculation it is assumed that the driver's eye is 3.5 feet above the ground and that the object to be seen is 3.5 feet above the ground of the intersecting road.

Intersections and driveways should, at a minimum, meet the SSD requirements; however, it is desirable to achieve the ISD whenever possible.

The standards for evaluating SSD and ISD follow the methodology in AASHTO's *A Policy on Geometric Design of Highways and Streets* (2011).

The site driveway on South Barlow Road was evaluated to determine if sight distances were met.

Based on a posted speed of 55 MPH and a grade near the intersection of less than 3%, the recommended SSD from the 2011 AASHTO manual is 495 feet for vehicles in both the northbound and southbound directions. The ISD calculation was based on Case B1 of the 2011 AASHTO manual. The design vehicle used was a combination truck with a base time gap of 11.5 seconds. Based on these assumptions the recommended ISD is 930 feet for a left turn and 850 feet for a right turn.

- The available stopping sight distance for northbound vehicles on South Barlow Road as they approach the driveway is over 1,000 feet. This extends beyond the recommended 495 foot stopping sight distance.
- The available stopping sight distance for southbound vehicles on South Barlow Road as they approach the driveway is over 1,000 feet. This exceeds the recommended 495-foot stopping sight distance.
- The available intersection sight distance for a truck making a right turn from the driveway onto South Barlow Road extends beyond 1,000 feet to the north. This meets the minimum of 850 feet for the right turn from the minor road (Case B-2, AASHTO).
- The available intersection sight distance for a truck making a left turn from the site driveway onto South Barlow Road is more than 1,000 feet to the north and south. The minimum intersection sight distance is recommended to be 930 feet for a truck making a left turn from the minor road (Case B-1, AASHTO). The available intersection sight distance meets the requirement.

The sight distance standards for dump trucks are met for this intersection.

## 5.2 TURNING MOVEMENTS

The site driveway was evaluated for the ability of a gravel truck to make the necessary turns. The evaluation looked at movements made by a single unit dump truck and dump truck with a pup trailer. The results illustrate that the site driveway, as currently constructed, can accommodate the necessary truck movements.

## 5.3 ROAD CAPACITY

The roadways along the haul route were evaluated to determine if there is enough capacity to handle the trips associated with the area proposed for rezoning, which will be consistent with existing background trips from the Cadman operations traffic from this site resulting from this approval.

The estimated capacity of a roadway is based on the *Highway Capacity Manual* where the theoretical maximum number of vehicles that can travel on roadway section per lane per hour is adjusted per physical characteristics of the roadway such as lane width, grade, speed, the presence of stop signs or traffic signals, and horizontal alignment. A typical level roadway at free-flow speeds can handle 3,200 cars per hour per lane as per the *Highway Capacity Manual*; however, based on regional characteristics, the recommended practice to use 1,750 vehicles per hour per lane and make the necessary adjustments as per roadway characteristics.

South Barlow Road has a through volume of 315 for the northbound direction and 477 for southbound direction for the year 2033 with the proposed aggregate extraction operation. The volume to capacity ratio (as calculated from SYNCHRO) for South Barlow Road is 0.01 for the northbound and 0.28 for the southbound direction. This means that 1% and 28% of the capacity is used, respectively. Therefore, the roadways have sufficient capacity to handle the traffic from the proposed operation.

#### 5.4 HORIZONTAL ALIGNMENT

South Barlow Road along the haul route was evaluated for roadway alignment to identify any significant safety or operational concerns that could be created or perpetuated by the added truck traffic.

Specifically, the roadway was evaluated to ensure adequate lane widths for trucks, paying significant attention to the off-tracking of a truck with a pup trailer. The existing lane widths are sufficient to handle the truck traffic along South Barlow Road.

### 6.0 TPR ANALYSIS (GOAL 12 EVALUATION)

The subject site is currently zoned Exclusive Farm Use (EFU). The applicant is requesting a zone map amendment to add a Mineral Aggregate Overlay (MAO) to support the proposed aggregate extraction of this site. The Transportation Planning Rule requires that the zone map amendment results in no significant effect on the adjacent transportation system.

The reasonable worst-case trip generation allowed under the current EFU zone designation is based upon the type and intensity of uses allowed in this street under the County zoning and development ordinance was determined using zoning types that are allowed within Clackamas County code. Table 5 depicts specifically allowed use and associated trip generation under current EFU zoning.

TABLE 5: EFU ZONING POTENTIAL TRIP GENERATION-PM PEAK HOUR

Allowed EFU Uses	ITE Lane Use Category	Size	Trip Generation Rate	PM Peak Hour Trips
Winery with Restaurant	931-Quality Restaurant	9,000 sf	7.8 trips/ksf	70
Golf Driving Range	432-Golf Driving Range	39 tees	1.25 trips/tee	49
Day Care Center	565 Day Care Center	2,000 sf	11.12 trips/ksf	22
<b>Total</b>				<b>141</b>

The proposed MAO zone amendment is anticipated to include the current production of not more than 3 million tons per year with no more than 120 trips in the PM peak hour.

The proposed zone map amendment will generate fewer trips than the worst-case trip generation associated with other EFU uses allowed under County code. Therefore, the TPR requirement of “no significant offset” under 660-12-0060 is met.

## 7.0 CODE COMPLIANCE

As per the analysis included within the report, the application can be found to meet all applicable traffic analysis code criteria as described in the following:

### GOAL 5 (OAR 660-23-180(5)(b)(B))

Goal 5 (OAR 660-23-180(5)(b)) “(B) Potential conflicts to local roads used for access and egress to the mining site within one mile of the entrance of the mining site unless a greater distance is necessary in order to include the intersection with the nearest arterial identified in the local transportation plan. Conflicts shall be determined based on clear and objective standards regarding sight distances, road capacity, cross-section elements, horizontal and vertical alignment, and similar items in the transportation plan and implementing ordinances. Such standards for trucks associated with the mining operation shall be equivalent to standards for other trucks of equivalent size, weight, and capacity that haul other materials;” OAR 660-23-180(5)(b)(B)

**The analysis evaluated the sight distance, intersection geometry, road capacity, and horizontal elements. All elements meet applicable standard either by existing conditions or by proposed mitigation to improve existing conditions.**

### GOAL 12, TPR (OAR 660-12-0060)

Goal 12, TPR (OAR 660-12-0060 (1)) requires that a local government ensures that an amendment to a functional plan, an acknowledged comprehensive plan, or a land use regulation (including a zoning map) does not significantly affect an existing or planned transportation facility. A plan or land use amendment significantly affects a transportation facility if it would:

“(a) Change the functional classification of an existing or planned transportation facility (exclusive of correction of map errors in an adopted plan);

**The traffic from the proposed plan amendment and use will not change the functional classification of any existing or planned transportation facilities.**

(b) Change standards implementing a functional classification system; or

**The traffic from the proposed plan amendment will not change the standard implementing a functional classification system.**

(c) Result in any of the effects listed in paragraphs (A) through (C) of this subsection based on projected conditions measured at the end of the planning period identified in the adopted TSP. As part of evaluating projected conditions, the amount of traffic projected to be generated within the area of the amendment may be reduced if the amendment includes an enforceable, ongoing requirement that would demonstrably limit traffic generation, including, but not limited to, transportation demand management. This reduction may diminish or completely eliminate the significant effect of the amendment.

(A) Types or levels of travel or access that are inconsistent with the functional classification of an existing or planned transportation facility;

**The traffic from the proposed plan amendment and use will not result in levels of travel or access that are inconsistent with the functional classification of an existing or planned transportation facility.**

(B) Degrade the performance of an existing or planned transportation facility such that it would not meet the performance standards identified in the TSP or comprehensive plan; or

**The traffic from the proposed plan amendment and use will not degrade the performance of any existing or planned transportation facility to below mobility standards.**

(C) Degrade the performance of an existing or planned transportation facility that is otherwise projected to not meet the performance standards identified in the TSP or comprehensive plan.” OAR 660-12-0060(1)

**This criterion is not applicable as none of the studied intersections have been identified to not meet the mobility standards.**

## 8.0 CONCLUSION

This analysis includes the following findings:

- Highway 99E at South Barlow Road currently does not meet the adopted mobility standard which is expected to continue through Clackamas County’s Transportation System Plan planning horizon. Oregon Highway Plan Action 1F.5 defines the mobility standard as no further degradation for this intersection. Since the application requests authorization of a minable inventory of aggregate resource to replace depleted resources at the facility, traffic associated with the application of the MAO overlay from the proposed aggregate extraction operation does not modify the volume-to-capacity

ratios beyond existing background conditions, thereby meeting ODOT mobility targets as defined in the Oregon Highway Plan.

- The existing site driveway will meet the mobility standard for the year of opening and through the end of Clackamas County Transportation System Plan planning horizon with the comprehensive plan amendment.
- The existing horizontal alignment of all roadways can accommodate truck traffic consistent with applicable standards under Goal 5.
- The application was found to meet all applicable traffic analysis code criteria.
- TPR has been demonstrated to be met for the proposed zone change and comprehensive plan amendment.



Cadman





# PROPERTY MAP

CANBY  
CLACKAMAS COUNTY, OREGON

Canby Property Boundary

Pit Boundary

## Subject Parcels

Cha

Lapp

Blumenkron

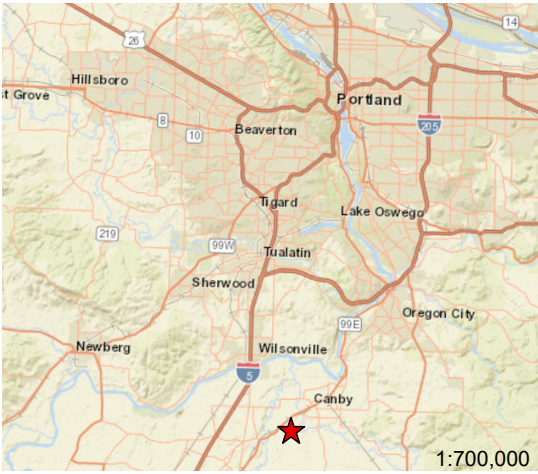
Lee

VanPelt

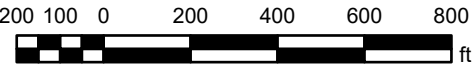
Yoder

Cadastral

Note:



Scale:



DESIGNED:

DRAWN: DS

CHECKED:

LOCATION: CLACKAMAS COUNTY

REGION: CANADA

FLIGHT DATE: 2016- 07- 14

PRINT DATE:

DATE: 2018- 10- 22



Map No.

1

**CADMAN**  
HEIDELBERGCEMENT Group

SUSTAINABLE RESOURCE  
DEVELOPMENT REGION CANADA  
8955 SHAUGHNESSY STREET  
VANCOUVER, BC, V6P 3Y7

Service Layer Credits: World Imagery: Source: Esri, DigitalGlobe, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, and the GIS User Community  
World Boundaries and Places: Esri, HERE, Garmin, © OpenStreetMap contributors, and the GIS user community  
World Street Map: Sources: Esri, HERE, Garmin, USGS, Intermap, INCREMENT P, NRCan, Esri Japan, METI, Esri China (Hong Kong), Esri Korea, Esri (Thailand), NGCC, © OpenStreetMap contributors, and the GIS User Community



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CRASH DATA SUMMARY

HWY 99E @ Barlow Rd

YEAR	PDO	INJURY	FATAL	HEAD	REAR	SIDE	TURN	ANGLE	OTHER	PED	BIKE	TOTAL
2013	5	3	1		1	3	4		1			9
2014	2	3					4	1				5
2015	3	5			3	1	3	1				8
2016	2	8			7	1	1	1				10
2017	3	6			3		4	1	1			9
TOTALS:	15	25	1	0	14	5	16	4	2	0	0	41

CHECK
OK
OK
OK
OK
OK
OK

Barlow Segment

YEAR	PDO	INJURY	FATAL	HEAD	REAR	SIDE	TURN	OTHER	PED	BIKE	TOTAL
2013	1					0		1	0	0	1
2014									0	0	0
2015		2				1			1	0	2
2016		1				1			0	0	1
2017	1					1			0	0	1
TOTALS:	2	3	0	0	0	3	0	1	1	0	5

CHECK
OK
OK
OK
OK
OK
OK

P.M. PEAK HOUR	Number of Years	ADT	AVG. ANNUAL MILES (MILLIONS)	AVG. YEARLY CRASHES	CRASH RATE/ MILLION MILES
4,443	5	40,000	14,600,000	8,200,000	0.56

Number of Years, n	ADT	AVG. ANNUAL MILES (MILLIONS)	AVG. YEARLY CRASHES	CRASH RATE/ MILLION MILES	CRASH RATE NORMALIZED OVER ONE MILE
1	5268	1922893.000	1000000.0	0.52	0.27
1	5417	1977314.500	0.0	0.00	0.00
1	5566	2031736.000	2000000.0	0.98	0.51
1	5716	2086157.500	1000000.0	0.48	0.25
1	5865	2140579.000	1000000.0	0.47	0.24
			Average	0.49	0.25











02/06/2019

TRANSPORTATION DATA SECTION - CRASH ANAYLYSIS AND REPORTING UNIT

CONTINUOUS SYSTEM CRASH LISTING

081: PACIFIC HIGHWAY EAST

Highway 081 ALL ROAD TYPES, MP 22.7 to 23.1 01/01/2013 to 12/31/2017, Both Add and Non-Add mileage

1 - 4 of 47 Crash records shown.

S D																													
SER#	P	R	S	W	DATE	COUNTY	RD#	FC	CONN#	RD	CHAR	INT-TYPE	SPCL USE																
INVEST	E	A	U	C	O DAY	CITY	COMPNT	FIRST	STREET	DIRECT	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	TRLR	QTY	MOVE	A S										
RD DPT	E	L	G	H	R TIME	URBAN AREA	MLG	TYP	SECOND	STREET	LOCTN	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G	E	LICNS	PED					
UNLOC?	D	C	S	L	K LAT	LONG	MILEPNT	LRS			(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR	ACT	EVENT	CAUSE
03056	N	N	N		07/27/2017	CLACKAMAS	1	16		STRGHT		N	N	CLR	S-STRGHT	01	NONE	9	STRGHT									29	
NO RPT					TH		MN	0		UN	(NONE)	UNKNOWN	N	DRY	REAR	N/A		N -S								000	00		
N					11P	CANBY UA	22.71			04			N	DARK	PDO	PSNGR	CAR		01	DRVR	NONE	00	Unk	UNK		000	000	00	
N					45 15 4.59	-122 43 12.04			008100100S00		(04)												UNK						
																02	NONE	9	STOP							011	00		
																N/A		N -S							000	000	00		
																SEMI	TOW		01	DRVR	NONE	00	Unk	UNK		000	000	00	
																							UNK						
00722	N	N	N	N	02/14/2016	CLACKAMAS	1	16		STRGHT		N	N	CLD	S-STRGHT	01	NONE	0	STRGHT								13		
STATE					SU		MN	0		UN	(NONE)	UNKNOWN	N	WET	SS-O	PRVTE		S -N								000	00		
N					6P	CANBY UA	22.74			06			N	DARK	INJ	PSNGR	CAR		01	DRVR	NONE	63	M	SUSP		045	000	13	
N					45 15 3.51	-122 43 13.67			008100100S00		(04)												OR<25						
																02	NONE	0	STRGHT							000	00		
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																PSNGR	CAR		01	DRVR	NONE	28	M	OR-Y		000	000	00	
																							OR<25						
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																02	NONE	0	STRGHT							000	00		
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																03	NONE	0	STRGHT							000	00		
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																PSNGR	CAR		01	DRVR	INJC	51	F	OR-Y		000	000	00	
																							OR<25						
																03	NONE	0	STRGHT							000	00		
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																PSNGR	CAR		02	PSNG	INJC	12	M			000	000	00	
03832	N	N	N	N	09/17/2017	CLACKAMAS	1	16		STRGHT		N	N	CLR	S-1TURN	01	NONE	0	STRGHT								08		
STATE					SU		MN	0		UN	(NONE)	UNKNOWN	N	DRY	TURN	PRVTE		N -S								000	00		
N					12A	CANBY UA	22.75			03			N	DARK	INJ	PSNGR	CAR		01	DRVR	INJC	30	M	OR-Y		000	000	00	
N					45 15 3.16	-122 43 14.22			008100100S00		(04)												OR<25						
																02	NONE	0	U-TURN							000	00		
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02244	N	N	N	N	06/09/2015	CLACKAMAS	1	16		STRGHT		N	N	CLR	S-1STOP	01	NONE	0	STRGHT								29,22		
STATE					TU		MN	0		UN	(NONE)	UNKNOWN	N	DRY	REAR	PRVTE		N -S								000	22		
N					5P	CANBY UA	22.84			04			N	DAY	INJ	PSNGR	CAR		01	DRVR	INJC	35	M	OTH-Y		026	000	29	
N					45 15 .03	-122 43 19.12			008100100S00		(04)												N-RES						
																02	NONE	0	STOP							011	00		
																PRVTE		N -S								000	00		
																PSNGR	CAR		01	DRVR	INJC	43	F	OR-Y		000	000	00	
																							OR<25						

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02/06/2019

TRANSPORTATION DATA SECTION - CRASH ANAYLYSIS AND REPORTING UNIT

CONTINUOUS SYSTEM CRASH LISTING

081: PACIFIC HIGHWAY EAST

Highway 081 ALL ROAD TYPES, MP 22.7 to 23.1 01/01/2013 to 12/31/2017, Both Add and Non-Add mileage

10 - 13 of 47 Crash records shown.

S D																																					
SER#	P	R	S	W	DATE	COUNTY	RD#	FC	CONN#	RD	CHAR	INT-TYPE		SPCL USE																							
INVEST	E	A	U	C	O	DAY	CITY	COMPNT	FIRST	STREET	DIRECT	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	TRLR	QTY	MOVE	A S																	
RD DPT	E	L	G	H	R	TIME	URBAN AREA	MLG	TYP	SECOND	STREET	LOCTN	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G	E	LICNS	PED												
UNLOC?	D	C	S	L	K	LAT	LONG	MILEPNT	LRS			(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR	ACT	EVENT	CAUSE							
03170	N	N	N			08/05/2015	CLACKAMAS	1	16			INTER	CROSS	N		CLR	S-1STOP	01	NONE	0	STRGHT																
NONE						WE		MN	0			UN		TRF	SIGNAL	N	UNK	REAR	PRVTE	UN-UN									000		00	29					
N						1P	CANBY UA	22.89				06	2		N	DAY	PDO	UNKNOWN		01	DRVR	NONE	00	F	UNK		026	000		29							
N						45 14 58.28	-122 43 21.83			008100100S00														OR<25													
																			02	NONE	0	STOP													011		00
																			PRVTE	UN-UN	01	DRVR	NONE	82	M	OR-Y	000	000		00							
																			PSNGR	CAR					OR<25												
06187	Y	N	N			12/05/2016	CLACKAMAS	1	16			INTER	CROSS	N		RAIN	S-1STOP	01	NONE	0	STRGHT													124	01,29		
NONE						MO		MN	0			UN		TRF	SIGNAL	N	WET	REAR	PRVTE	N-S									000	124	00						
N						12P	CANBY UA	22.89				06	2		N	DAY	INJ	PSNGR	CAR	01	DRVR	NONE	28	M	OR-Y	047,026	000		01,29								
N						45 14 58.28	-122 43 21.83			008100100S00														OR<25													
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																			02	NONE	0	STOP													012		00
																			PRVTE	N-S	02	PSNG	INJC	31	F		000	000		00							
																			PSNGR	CAR																	
																			02	NONE	0	STOP													012		00
																			PRVTE	N-S	03	PSNG	NO<5	03	F		000	000		00							
																			PSNGR	CAR																	
01558	N	N	N	N	N	05/06/2013	CLACKAMAS	1	06			INTER	CROSS	N		CLR	FIX OBJ	01	NONE	0	STRGHT													055	26		
COUNTY						MO		MN	0			N		TRF	SIGNAL	N	DRY	FIX	PRVTE	NE-SW									007	055	00						
N						11A		22.89				05	2		N	DAY	INJ	PSNGR	CAR	01	DRVR	INJC	61	M	OR-Y	081	000		26								
N						45 14 58.2806759	-122 43 21.826092			008100100S00														OR<25													
																			01	NONE	0	STRGHT													007	055	00
																			PRVTE	NE-SW	02	PSNG	INJC	16	M		000	000		00							
																			PSNGR	CAR																	
03903	N	N	N			08/26/2016	CLACKAMAS	1	16			INTER	CROSS	N		CLR	S-1STOP	01	NONE	0	STRGHT													013	29		
NO RPT						FR		MN	0			N		TRF	SIGNAL	N	DRY	REAR	PRVTE	NE-SW									000		00						
N						9A	CANBY UA	22.89				06	2		N	DAY	INJ	PSNGR	CAR	01	DRVR	NONE	29	M	OR-Y	026	000		29								
N						45 14 58.28	-122 43 21.83			008100100S00														OR>25													
																			02	NONE	0	STOP													011	013	00
																			PRVTE	NE-SW	01	DRVR	NONE	41	M	OR-Y	000	000		00							
																			PSNGR	CAR					OR<25												
																			03	NONE	0	STOP													022		00
																			PRVTE	NE-SW	01	DRVR	INJC	34	F	OR-Y	000	000		00							
																			PSNGR	CAR					OR<25												
																			03	NONE	0	STOP													022		00
																			PRVTE	NE-SW	02	PSNG	NO<5	02	M		000	000		00							
																			PSNGR	CAR																	

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081: PACIFIC HIGHWAY EAST

Highway 081 ALL ROAD TYPES, MP 22.7 to 23.1 01/01/2013 to 12/31/2017, Both Add and Non-Add mileage

37 - 41 of 47 Crash records shown.

SER#	P	R	S	W	DATE	COUNTY	RD#	FC	CONN#	RD	CHAR	INT-TYPE	SPCL	USE																	
INVEST	E	A	U	C	O	DAY	CITY	COMPNT	FIRST	STREET	DIRECT	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	TRLR	QTY	MOVE												
RD DPT	E	L	G	H	R	TIME	URBAN AREA	MLG	TYP	SECOND	STREET	LOCTN	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G	E	LICNS	PED						
UNLOC?	D	C	S	L	K	LAT	LONG	MILEPNT	LRS			(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR	ACT	EVENT	CAUSE	
00179	N	N	N	N	N	01/14/2014	CLACKAMAS	1	06		INTER	CROSS	N	N	FOG	O-1 L-TURN	01	NONE	0	STRGHT										02	
STATE						TU		MN	0		CN		TRF	SIGNAL	N	DRY	TURN	PRVTE	S -N									000	00		
N						7A		22.89			04	2		N	DLIT	INJ		PSNGR	CAR		01	DRVR	INJC	49	M	OR-Y		000	000	00	
N						45 14 58.28	-122 43 21.8261279			008100100S00														OR<25							
																		02	NONE	0	TURN-L										
																		PRVTE	N -NE									000	00		
																		PSNGR	CAR		01	DRVR	INJC	16	M	OR-Y		028,004	000	02	
02004	N	N	N			05/27/2014	CLACKAMAS	1	06		INTER	CROSS	N	N	CLR	O-1 L-TURN	01	NONE	0	STRGHT									02		
NONE						TU		MN	0		CN		TRF	SIGNAL	N	DRY	TURN	PRVTE	S -N									000	00		
N						5P		22.89			04	2		N	DAY	INJ		PSNGR	CAR		01	DRVR	INJC	23	F	OR-Y		000	000	00	
N						45 14 58.28	-122 43 21.83			008100100S00															OR<25						
																		01	NONE	0	STRGHT										
																		PRVTE	S -N									000	00		
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																		PRVTE	N -E									000	00		
																		PSNGR	CAR		01	DRVR	NONE	00	M	UNK		028,004	000	02	
																									UNK						
02333	N	N	N	N	N	06/14/2015	CLACKAMAS	1	16		INTER	CROSS	N	N	CLR	O-1 L-TURN	01	NONE	0	STRGHT									02		
STATE						SU		MN	0		CN		TRF	SIGNAL	N	DRY	TURN	PRVTE	S -N									000	00		
N						4P	CANBY UA	22.89			04	2		N	DAY	INJ		PSNGR	CAR		01	DRVR	INJB	24	M	OR-Y		000	000	00	
N						45 14 58.28	-122 43 21.83			008100100S00															OR<25						
																		01	NONE	0	STRGHT										
																		PRVTE	S -N									000	00		
																		PSNGR	CAR		02	PSNG	INJB	18	M			000	000	00	
																		02	NONE	0	TURN-L										
																		PRVTE	N -E									000	00		
																		PSNGR	CAR		01	DRVR	INJC	54	F	OR-Y		028,004	000	02	
																									OR<25						
02768	N	N	N			07/11/2017	CLACKAMAS	1	16		INTER	CROSS	N	N	CLR	O-1 L-TURN	01	NONE	0	STRGHT									02		
NO RPT						TU		MN	0		CN		TRF	SIGNAL	N	DRY	TURN	PRVTE	S -N									000	00		
N						2P	CANBY UA	22.89			04	2		N	DAY	INJ		PSNGR	CAR		01	DRVR	INJB	29	M	OR-Y		000	000	00	
N						45 14 58.3	-122 43 21.82			008100100S00															OR>25						
																		02	NONE	0	TURN-L										
																		PRVTE	N -NE									000	00		
																		PSNGR	CAR		01	DRVR	NONE	82	M	OR-Y		028,004	000	02	
																									OR<25						
03146	N	N	N	N	N	08/03/2017	CLACKAMAS	1	16		INTER	CROSS	N	N	CLR	ANGL-OTH	01	NONE	0	STRGHT									02		
STATE						TH		MN	0		CN		TRF	SIGNAL	N	DRY	ANGL	PRVTE	SW-NE									000	00		
N						10P	CANBY UA	22.89			04	2		N	DARK	INJ		PSNGR	CAR		01	DRVR	INJC	29	F	OR-Y		000	000	00	
N						45 14 58.31	-122 43 21.86			008100100S00															OR<25						
																		02	NONE	0	STRGHT										
																		PRVTE	S -N									000	00		
																		PSNGR	CAR		01	DRVR	NONE	29	F	OR-Y		028	000	02	
																									OR<25						

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02/06/2019

TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT

CONTINUOUS SYSTEM CRASH LISTING

081: PACIFIC HIGHWAY EAST

Highway 081 ALL ROAD TYPES, MP 22.7 to 23.1 01/01/2013 to 12/31/2017, Both Add and Non-Add mileage

42 - 46 of 47 Crash records shown.

SER#	P	R	S	W	DATE	COUNTY	RD#	FC	CONN#	RD	CHAR	INT-TYPE	SPCL USE																	
INVEST	E	A	U	C	O	DAY	CITY	COMPNT	FIRST STREET	DIRECT	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	TRLR	QTY	MOVE	A S											
RD DPT	E	L	G	H	R	TIME	URBAN AREA	MLG	TYP	SECOND STREET	LOCTN	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G	E	LICNS	PED						
UNLOC?	D	C	S	L	K	LAT	LONG	MILEPNT	LRS		(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR	ACT	EVENT	CAUSE	
04197	N	N	N	N	N	10/09/2017	CLACKAMAS	1	16		INTER	CROSS	N	N	CLD	O-1 L-TURN	01	NONE	0	STRGHT									27,02,08	
STATE						MO		MN	0		CN		TRF	SIGNAL	N	DRY	TURN	PRVTE	S	-N							000		00	
N						7A	CANBY UA	22.89			04	2		N	DAY	INJ	PSNGR	CAR		01	DRVR	INJC	39	M	OR-Y		000	000	00	
N						45 14 58.29	-122 43 21.83			008100100S00													OR<25							
																	02	NONE	0	TURN-L							000		00	
																	PSNGR	CAR		01	DRVR	INJC	23	F	OR-Y		028,004	000	27,02,08	
																							OR<25							
04238	N	N	N			10/12/2017	CLACKAMAS	1	16		INTER	CROSS	N	N	RAIN	ANGL-OTH	01	NONE	9	STRGHT									04	
NONE						TH		MN	0		CN		TRF	SIGNAL	N	WET	ANGL	N/A	S	-N							000		00	
N						5A	CANBY UA	22.89			04	2		N	DLIT	PDO	PSNGR	CAR		01	DRVR	NONE	00	Unk	UNK		000	000	00	
N						45 14 58.31	-122 43 21.87			008100100S00													UNK							
																	02	NONE	9	STRGHT							000		00	
																	N/A	SW-NE		01	DRVR	NONE	00	Unk	UNK		000	000	00	
																	PSNGR	CAR					UNK							
00990	N	N	N			03/22/2013	CLACKAMAS	1	06		STRGHT		Y	N	CLR	O-1STOP	01	NONE	0	BACK									10	
NONE						FR		MN	0		UN	(NONE)	TRF	SIGNAL	N	DRY	BACK	PRVTE	NE-SW								000		00	
N						8A		22.91			06			N	DAWN	PDO	PSNGR	CAR		01	DRVR	NONE	39	M	OR-Y		011	000	10	
N						45 14 57.7464359	-122 43 22.7816759			008100100S00			(04)											OR>25						
																	02	NONE	0	STOP							011		00	
																	PRVTE	SW-NE		01	DRVR	NONE	00	M	UNK		000	000	00	
																	PSNGR	CAR					OR<25							
03205	N	N	N			08/29/2013	CLACKAMAS	1	06		STRGHT		N	N	UNK	S-STRGHT	01	NONE	0	STRGHT									13	
NONE						TH		MN	0		UN	(NONE)	UNKNOWN		N	WET	SS-O	PRVTE	N	-S							000		00	
N						7A		22.95			03			N	DAY	PDO	PSNGR	CAR		01	DRVR	NONE	00	M	OR-Y		045	000	13	
N						45 14 56.439492	-122 43 24.8829599			008100100S00			(04)											UNK						
																	02	NONE	0	STRGHT								000		00
																	PRVTE	N	-S		01	DRVR	NONE	28	F	OR-Y		000	000	00
																	PSNGR	CAR					OR<25							
02759	N	N	N	N	N	08/10/2013	CLACKAMAS	1	06		STRGHT		N	N	CLR	O-STRGHT	01	NONE	0	STRGHT									079,010	05
STATE						SA		MN	0		UN	(NONE)	UNKNOWN		N	DRY	SS-M	PRVTE	S	-N							022	079,010	00	
Y						8P		23.00			04			N	DAY	FAT	PSNGR	CAR		01	DRVR	KILL	21	M	OR-Y		080	000	05	
N						45 14 54.715668	-122 43 27.5973599			008100100S00			(04)											OR<25						
																	02	NONE	1	STRGHT								000		00
																	PRVTE	N	-S		01	DRVR	NONE	54	M	OTH-Y		000	000	00
																	PSNGR	CAR					N-RES							
																	03	NONE	0	STRGHT								022		00
																	PRVTE	N	-S		01	DRVR	INJC	57	M	OR-Y		000	000	00
																	PSNGR	CAR					OR<25							

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Cadman



Intersection: 1: S Barlow Rd @ Pacific HWY 99E										City: Canby, Or													
Counter: Sandow Engineering										Date: Sept 12, 2017													
Total of All Vehicles																							
Time Period		Southbound				Westbound				Northbound				Eastbound				15 Minute Volume	Hourly Volume	Pedestrians			
		Right	Thru	Left	Approach Total	Right	Thru	Left	Approach Total	Right	Thru	Left	Approach Total	Right	Thru	Left	Approach Total			SB	WB	NB	EB
15:30	15:45	12	46	102	160	67	144	50	261	24	16	5	45	2	159	9	170	636	0	0	0	0	
15:45	16:00	6	73	78	156	61	175	39	263	32	20	4	56	7	171	5	176	639	0	0	0	0	
16:00	16:15	6	73	89	168	54	199	44	293	30	20	4	51	7	173	5	166	639	0	0	0	0	
16:15	16:30	5	40	79	124	62	212	44	312	22	28	1	51	2	163	7	172	659	2561	0	0	0	
16:30	16:45	11	45	95	151	64	208	41	313	22	21	1	44	0	193	6	199	707	2632	0	0	0	
16:45	17:00	2	41	82	125	54	194	37	285	18	23	1	42	0	233	3	236	688	2681	0	0	0	
17:00	17:15	2	51	93	146	62	174	34	270	15	26	3	44	0	197	6	203	663	2777	0	0	0	
17:15	17:30	1	45	75	121	52	185	44	281	23	22	1	46	3	169	8	180	628	2686	0	0	0	
17:30	17:45	2	59	100	161	54	168	41	263	21	24	0	45	0	160	7	167	636	2615	0	0	0	
17:45	18:00	2	38	82	122	34	149	30	213	18	24	2	44	1	149	7	157	536	2463	0	0	0	
18:00	18:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1800	0	0	0	0	
18:15	18:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1172	0	0	0	0	
Count Period Total		49	440	866		568	1799	409		225	224	19		15	1744	61		6419	0	0	0	0	
PM Peak Hour Count Summary																							
Peak Volumes		Southbound				Westbound				Northbound				Eastbound				Approach		Pedestrians			
		Right	Thru	Left	Approach	Right	Thru	Left	Approach	Right	Thru	Left	Approach	Right	Thru	Left	Approach	SB	WB	NB	EB		
PHF		20	177	349	546	246	788	156	1190	77	98	6	181	2	786	22	810	2727	0	0	0	0	
Trucks		1	4	0	0.90	0.93	0.93	0.89	0.92	0.88	0.88	0.50	0.89	0.25	0.84	0.79	0.86	0.96					
% Trucks		5%	2%	0%		0%	1%	1%		1%	1%	0%		0%	2%	14%							

[illegible]

1: S Barlow Rd @ Pacific HWY 99E

## Pedestrians and Cars

Time Period		Southbound				Westbound				Northbound				Eastbound				15 Minute Volume	Hourly Volume
		Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left		
3:30 PM	12		46	95		65	142	47		24	16	5			1	144	6	603	
3:45 PM	5		44	77		59	171	38		29	19	4			0	161	5	612	
4:00 PM	6		26	77		52	185	46		29	20	1			7	140	2	591	
4:15 PM	5		38	78		65	205	42		21	26	1			2	159	5	647	
4:30 PM	10		45	92		61	200	38		20	19	0			0	184	6	675	
4:45 PM	2		41	81		61	189	35		17	23	1			0	227	2	2525	
5:00 PM	1		51	92		61	167	34		15	26	3			0	193	5	2585	
5:15 PM	1		45	74		49	180	43		22	22	1			3	166	8	2642	
5:30 PM	1		57	100		54	162	39		20	23	0			0	155	7	2609	
5:45 PM	2		36	82		33	145	30		18	24	2			1	142	4	2552	
6:00 PM																		2399	
Total	0	45	429	848		553	1746	392		0	215	218	18		0	14	1671	50	0
Peak Hour	0	3	93	182		0	87	307	69		0	38	47	2	0	1	297	11	1137

## Trucks

Time Period	Southbound			Westbound			Northbound			Eastbound			15 Minute Volume
	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	
3:30 PM	0	7	3	2	2	3	0	0	0	1	15	3	33
3:45 PM	1	4	1	2	4	1	3	0	0	0	10	0	27
4:00 PM	0	1	3	2	5	3	1	0	0	0	10	1	26
4:15 PM	0	2	1	1	7	2	1	2	0	0	4	2	22
4:30 PM	0	0	3	3	8	3	2	2	0	0	9	0	32
4:45 PM	1	0	1	0	5	2	0	1	0	0	6	1	16
5:00 PM	1	0	1	0	7	0	0	0	0	0	4	1	16
5:15 PM	0	0	1	3	5	0	0	0	0	0	3	0	15
5:30 PM	1	2	0	0	6	2	1	0	0	0	5	0	14
5:45 PM	0	2	0	1	4	0	0	0	0	0	7	3	18
6:00 PM													64
6:15 PM													17
Total	4	11	18	15	53	17	10	6	1	1	73	11	0
Peak Hour	1	4	0	1	10	2	1	1	0	0	12	3	35

## Bikes

[illegible]

### Pedestrians

[illegible]

Existing 2019 PM Volumes

		561		376	
		R	T	L	PED
1		21	182	358	0
1: S Barlow Rd @ Pacific HWY 99E					
835	Ped	0			252 R
23	L				808 T
806	T				160 L
831	R	2			0 Ped
		0	6	101	79
		Ped	L	T	R
				344	186

EDIT Hightmited	
Base Year	2019
Target Year	2025
Years of Growth	6
Barlow Growth rate	0.025
Growth Factor	1.15
Hwy 99 Growth rate	0.01
Growth Factor	1.06

1220

1243

		R	T	L	PED
1		0	343	0	0
2: S Barlow Rd @ Site Driveway					
	Ped	0			0 R
0	L	0			0 T
	T	0			0 L
0	R	0			0 Ped
		0	0	182	0
		Ped	L	T	R
				343	182

0

0

2033 Background volumes

		757		450	
		R	T	L	PED
1		28	246	483	0
1: S Barlow Rd @ Pacific HWY 99E					
958	Ped	0			287 R
26	L				921 T
919	T				182 L
947	R	2			0 Ped
		0	8	136	107
		Ped	L	T	R
				430	251

1390.8

1508.79

EDIT Hightmited	
Base Year	2019
Target Year	2033
Years of Growth	14
Barlow Growth rate	0.025
Growth Factor	1.35
Hwy 99 Growth rate	0.01
Growth Factor	1.14

		430		251	
		R	T	L	PED
1		0	430	0	0
2: S Barlow Rd @ Site Driveway					
	Ped	0			0 R
0	L	0			0 T
	T	0			0 L
0	R	0			0 Ped
		0	0	251	0
		Ped	L	T	R
				430	251

0

0

2025 Existing Splits

		561				376			
		R	T	L	PED	R	T	L	PED
		1	21	182	358	0			
835	Ped	0				252	R		
	L	23	1: S Barlow Rd @ Pacific				808	T	
	T	806	HWY 99E				160	L	
831	R	2							0 Ped
		0	6	101	79				
		Ped	L	T	R				
		344				186			

1220

1243

		0				0			
		R	T	L	PED	R	T	L	PED
		1	0	0	0	0			
0	Ped	0				0	R		
	L	0	2: S Barlow Rd @ Site				0	T	
	T	0	Driveway				0	L	
0	R	0							0 Ped
		0	0	0	0				
		Ped	L	T	R				
		0				0			

0

0

Development Trips:

Development trips

		24				33			
		R	T	L	PED	R	T	L	PED
		1	0	24	0	0			
2	Ped	0				0	R		
	L	0	1: S Barlow Rd @ Pacific				0	T	
	T	0	HWY 99E				21	L	
2	R	2							0 Ped
		0	2	33	26				
		Ped	L	T	R				
		47				61			

21

26

		47				61			
		R	T	L	PED	R	T	L	PED
		1		0	47	0			
0	Ped	0				61	R		
	L					0	T		
	T	0				6	L		
0	R								0 Ped
		0			0	6			
		Ped	L	T	R				
		6				6			

67

53

2033 Build Volumes

		781		483			
		R	T	L	PED		
1		28	270	483	0		
		1: S Barlow Rd @ Pacific HWY 99E				287	R
959.5	Ped	0				921	T
	L	26				203	L
	T	919				0	Ped
949.3	R	4					
		0	10	169	133		
		Ped	L	T	R		
		477				312	

1412

1535

		477		312			
		R	T	L	PED		
1		0	430	47	0		
		2: S Barlow Rd @ Site Driveway				61	R
0	Ped	0				0	T
	L	0				6	L
	T	0				0	Ped
0	R	0					
		0	0	251	6		
		Ped	L	T	R		
		436				257	

67





















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Cadman



Lanes, Volumes, Timings  
3: HWY 99 & Barlow Rd













02/06/2019

												
Lane Group	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Traffic Volume (vph)	6	101	79	358	182	21	23	806	2	160	808	252
Future Volume (vph)	6	101	79	358	182	21	23	806	2	160	808	252
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	0		0	0		0	275		115	285		230
Storage Lanes	0		0	0		0	1		1	1		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.943			0.995				0.850			0.850
Flt Protected		0.998			0.969		0.950			0.950		
Satd. Flow (prot)	0	1631	0	0	1665	0	1662	2969	1444	1630	3023	1473
Flt Permitted		0.985			0.668		0.950			0.950		
Satd. Flow (perm)	0	1610	0	0	1148	0	1662	2969	1444	1630	3023	1473
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		44			2				95			263
Link Speed (mph)		55			35			55			55	
Link Distance (ft)		1272			1716			3436			1917	
Travel Time (s)		15.8			33.4			42.6			23.8	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	0%	1%	1%	0%	4%	1%	0%	12%	3%	2%	10%	1%
Adj. Flow (vph)	6	105	82	373	190	22	24	840	2	167	842	263
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	193	0	0	585	0	24	840	2	167	842	263
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2	1	1	2	1
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right	Left	Thru	Right
Leading Detector (ft)	20	100		20	100		20	100	20	20	100	20
Trailing Detector (ft)	0	0		0	0		0	0	0	0	0	0
Detector 1 Position(ft)	0	0		0	0		0	0	0	0	0	0
Detector 1 Size(ft)	20	6		20	6		20	6	20	20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Prot	NA	Perm	Prot	NA	Perm
Protected Phases		4			8		5	2		1	6	

# Lanes, Volumes, Timings

## 3: HWY 99 & Barlow Rd

02/06/2019

												
Lane Group	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NET	NER	SWL	SWT	SWR
Permitted Phases	4			8					2			6
Detector Phase	4	4		8	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	37.5	37.5		23.0	23.0		9.5	23.0	23.0	9.5	23.0	23.0
Total Split (s)	63.0	63.0		63.0	63.0		9.6	39.6	39.6	17.4	47.4	47.4
Total Split (%)	52.5%	52.5%		52.5%	52.5%		8.0%	33.0%	33.0%	14.5%	39.5%	39.5%
Maximum Green (s)	58.5	58.5		58.5	58.5		5.1	35.1	35.1	12.9	42.9	42.9
Yellow Time (s)	4.0	4.0		4.0	4.0		3.5	4.0	4.0	3.5	4.0	4.0
All-Red Time (s)	0.5	0.5		0.5	0.5		1.0	0.5	0.5	1.0	0.5	0.5
Lost Time Adjust (s)		-0.5			0.0		-0.5	-0.5	-0.5	-0.5	-0.5	-0.5
Total Lost Time (s)		4.0			4.5		4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None		None	None		None	Max	Max	None	Max	Max
Walk Time (s)	7.0	7.0		7.0	7.0			7.0	7.0		7.0	7.0
Flash Dont Walk (s)	26.0	26.0		0.0	0.0			11.0	11.0		11.0	11.0
Pedestrian Calls (#/hr)	0	0		0	0			0	0		0	0
Act Effect Green (s)		59.0			58.5		5.6	35.6	35.6	13.4	47.2	47.2
Actuated g/C Ratio		0.49			0.49		0.05	0.30	0.30	0.11	0.39	0.39
v/c Ratio		0.24			1.04		0.31	0.95	0.00	0.92	0.71	0.36
Control Delay		14.2			81.3		66.2	62.9	0.0	101.3	35.5	4.6
Queue Delay		0.0			0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay		14.2			81.3		66.2	62.9	0.0	101.3	35.5	4.6
LOS		B			F		E	E	A	F	D	A
Approach Delay		14.2			81.3			62.9			37.7	
Approach LOS		B			F			E			D	

### Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Natural Cycle: 100

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.04

Intersection Signal Delay: 52.4







Intersection LOS: D

Intersection Capacity Utilization 92.2%

ICU Level of Service F

Analysis Period (min) 15





















Splits and Phases: 3: HWY 99 & Barlow Rd

		
Ø1	Ø2	Ø4
17.4 s	39.6 s	63 s
		
Ø5	Ø6	Ø8
9.6 s	47.4 s	63 s

# HCM Signalized Intersection Capacity Analysis






















## 3: HWY 99 & Barlow Rd

02/06/2019

												
Movement	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Traffic Volume (vph)	6	101	79	358	182	21	23	806	2	160	808	252
Future Volume (vph)	6	101	79	358	182	21	23	806	2	160	808	252
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)		4.0			4.5		4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor		1.00			1.00		1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.94			0.99		1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected		1.00			0.97		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)		1631			1665		1662	2969	1444	1630	3023	1473
Flt Permitted		0.98			0.67		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)		1609			1148		1662	2969	1444	1630	3023	1473
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	6	105	82	373	190	22	24	840	2	167	842	262
RTOR Reduction (vph)	0	23	0	0	1	0	0	0	1	0	0	161
Lane Group Flow (vph)	0	170	0	0	584	0	24	840	1	167	842	102
Heavy Vehicles (%)	0%	1%	1%	0%	4%	1%	0%	12%	3%	2%	10%	1%
Turn Type	Perm	NA		Perm	NA		Prot	NA	Perm	Prot	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8					2			6
Actuated Green, G (s)		58.5			58.5		3.1	36.9	36.9	12.9	46.7	46.7
Effective Green, g (s)		59.0			58.5		3.6	37.4	37.4	13.4	47.2	47.2
Actuated g/C Ratio		0.48			0.48		0.03	0.31	0.31	0.11	0.39	0.39
Clearance Time (s)		4.5			4.5		4.5	4.5	4.5	4.5	4.5	4.5
Vehicle Extension (s)		3.0			3.0		3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)		779			551		49	911	443	179	1171	570
v/s Ratio Prot							0.01	c0.28		c0.10	0.28	
v/s Ratio Perm		0.11			c0.51				0.00			0.07
v/c Ratio		0.22			1.06		0.49	0.92	0.00	0.93	0.72	0.18
Uniform Delay, d1		18.1			31.6		58.2	40.8	29.3	53.8	31.7	24.5
Progression Factor		1.00			1.00		1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2		0.1			55.2		7.5	16.0	0.0	48.0	3.8	0.7
Delay (s)		18.2			86.8		65.7	56.8	29.3	101.7	35.5	25.2
Level of Service		B			F		E	E	C	F	D	C
Approach Delay (s)		18.2			86.8			57.0			42.1	
Approach LOS		B			F			E			D	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			53.9				HCM 2000 Level of Service			D		
HCM 2000 Volume to Capacity ratio			1.00									
Actuated Cycle Length (s)			121.8				Sum of lost time (s)			12.5		
Intersection Capacity Utilization			92.2%				ICU Level of Service			F		
Analysis Period (min)			15									
c Critical Lane Group												













Lanes, Volumes, Timings  
3: HWY 99 & Barlow Rd

02/06/2019

												
Lane Group	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Traffic Volume (vph)	8	136	107	483	246	28	26	919	2	182	921	287
Future Volume (vph)	8	136	107	483	246	28	26	919	2	182	921	287
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	0		0	0		0	275		115	285		230
Storage Lanes	0		0	2		0	1		1	1		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.943			0.985				0.850			0.850
Flt Protected		0.998		0.950			0.950			0.950		
Satd. Flow (prot)	0	1631	0	3225	1662	0	1614	2969	1488	1630	3023	1473
Flt Permitted		0.987		0.950			0.950			0.950		
Satd. Flow (perm)	0	1613	0	3225	1662	0	1614	2969	1488	1630	3023	1473
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		35			7				104			288
Link Speed (mph)		55			35			55			55	
Link Distance (ft)		1272			1716			3436			1917	
Travel Time (s)		15.8			33.4			42.6			23.8	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	0%	1%	1%	0%	4%	1%	3%	12%	0%	2%	10%	1%
Adj. Flow (vph)	8	142	111	503	256	29	27	957	2	190	959	299
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	261	0	503	285	0	27	957	2	190	959	299
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		24			24			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2	1	1	2	1
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right	Left	Thru	Right
Leading Detector (ft)	20	100		20	100		20	100	20	20	100	20
Trailing Detector (ft)	0	0		0	0		0	0	0	0	0	0
Detector 1 Position(ft)	0	0		0	0		0	0	0	0	0	0
Detector 1 Size(ft)	20	6		20	6		20	6	20	20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Prot	NA		Prot	NA	Perm	Prot	NA	Perm
Protected Phases		4		3	8		5	2		1	6	

Lanes, Volumes, Timings  
3: HWY 99 & Barlow Rd

02/06/2019

												
Lane Group	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NET	NER	SWL	SWT	SWR
Permitted Phases	4								2			6
Detector Phase	4	4		3	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		8.5	5.0		8.0	5.0	5.0	8.0	5.0	5.0
Minimum Split (s)	37.5	37.5		13.0	23.0		13.0	23.0	23.0	13.0	23.0	23.0
Total Split (s)	43.6	43.6		15.0	58.6		13.0	39.4	39.4	17.0	43.4	43.4
Total Split (%)	37.9%	37.9%		13.0%	51.0%		11.3%	34.3%	34.3%	14.8%	37.7%	37.7%
Maximum Green (s)	39.1	39.1		10.5	54.1		8.0	34.9	34.9	12.0	38.9	38.9
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	0.5	0.5		0.5	0.5		1.0	0.5	0.5	1.0	0.5	0.5
Lost Time Adjust (s)		-0.5		-0.5	-0.5		-0.5	-0.5	-0.5	-0.5	-0.5	-0.5
Total Lost Time (s)		4.0		4.0	4.0		4.5	4.0	4.0	4.5	4.0	4.0
Lead/Lag	Lag	Lag		Lead			Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes			Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None		None	None		None	Max	Max	None	Max	Max
Walk Time (s)	7.0	7.0						7.0	7.0		7.0	7.0
Flash Dont Walk (s)	26.0	26.0						11.0	11.0		11.0	11.0
Pedestrian Calls (#/hr)	0	0						0	0		0	0
Act Effect Green (s)		19.1		11.0	34.1		8.5	35.5	35.5	12.5	47.6	47.6
Actuated g/C Ratio		0.20		0.12	0.36		0.09	0.37	0.37	0.13	0.50	0.50
v/c Ratio		0.74		1.34	0.47		0.19	0.86	0.00	0.88	0.63	0.34
Control Delay		43.5		205.2	25.3		45.5	37.7	0.0	80.6	22.3	4.1
Queue Delay		0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay		43.5		205.2	25.3		45.5	37.7	0.0	80.6	22.3	4.1
LOS		D		F	C		D	D	A	F	C	A
Approach Delay		43.5			140.2			37.9			26.2	
Approach LOS		D			F			D			C	

Intersection Summary

Area Type: Other

Cycle Length: 115

Actuated Cycle Length: 94.7

Natural Cycle: 120

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.34

Intersection Signal Delay: 56.6








Intersection LOS: E

Intersection Capacity Utilization 83.5%

ICU Level of Service E

Analysis Period (min) 15






















Splits and Phases: 3: HWY 99 & Barlow Rd

 Ø1	 Ø2	 Ø3	 Ø4
17 s	39.4 s	15 s	43.6 s
 Ø5	 Ø6	 Ø8	
13 s	43.4 s	58.6 s	

# HCM Signalized Intersection Capacity Analysis

## 3: HWY 99 & Barlow Rd

02/06/2019






















												
Movement	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Traffic Volume (vph)	8	136	107	483	246	28	26	919	2	182	921	287
Future Volume (vph)	8	136	107	483	246	28	26	919	2	182	921	287
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)		4.0		4.0	4.0		4.5	4.0	4.0	4.5	4.0	4.0
Lane Util. Factor		1.00		0.97	1.00		1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.94		1.00	0.98		1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected		1.00		0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)		1631		3225	1662		1614	2969	1488	1630	3023	1473
Flt Permitted		0.99		0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)		1613		3225	1662		1614	2969	1488	1630	3023	1473
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	8	142	111	503	256	29	27	957	2	190	959	299
RTOR Reduction (vph)	0	28	0	0	5	0	0	0	1	0	0	148
Lane Group Flow (vph)	0	233	0	503	280	0	27	957	1	190	959	151
Heavy Vehicles (%)	0%	1%	1%	0%	4%	1%	3%	12%	0%	2%	10%	1%
Turn Type	Perm	NA		Prot	NA		Prot	NA	Perm	Prot	NA	Perm
Protected Phases		4		3	8		5	2		1	6	
Permitted Phases	4								2			6
Actuated Green, G (s)		18.6		10.5	33.6		3.0	38.2	38.2	12.0	47.2	47.2
Effective Green, g (s)		19.1		11.0	34.1		3.5	38.7	38.7	12.5	47.7	47.7
Actuated g/C Ratio		0.20		0.11	0.35		0.04	0.40	0.40	0.13	0.49	0.49
Clearance Time (s)		4.5		4.5	4.5		5.0	4.5	4.5	5.0	4.5	4.5
Vehicle Extension (s)		3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)		315		362	579		57	1174	588	208	1474	718
v/s Ratio Prot				c0.16	0.17		0.02	c0.32		c0.12	0.32	
v/s Ratio Perm		c0.14							0.00			0.10
v/c Ratio		0.74		1.39	0.48		0.47	0.82	0.00	0.91	0.65	0.21
Uniform Delay, d1		37.0		43.4	25.0		46.2	26.4	17.9	42.1	18.8	14.3
Progression Factor		1.00		1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2		8.8		191.5	0.6		6.1	6.3	0.0	39.0	2.2	0.7
Delay (s)		45.8		234.9	25.6		52.3	32.6	17.9	81.2	21.0	15.0
Level of Service		D		F	C		D	C	B	F	C	B
Approach Delay (s)		45.8			159.2			33.2			27.7	
Approach LOS		D			F			C			C	

### Intersection Summary

HCM 2000 Control Delay	60.3	HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio	0.89		
Actuated Cycle Length (s)	97.8	Sum of lost time (s)	16.5
Intersection Capacity Utilization	83.5%	ICU Level of Service	E
Analysis Period (min)	15		
c Critical Lane Group			













Lanes, Volumes, Timings  
3: HWY 99 & Barlow Rd

02/06/2019

												
Lane Group	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Traffic Volume (vph)	9	150	120	483	258	28	26	919	4	193	921	287
Future Volume (vph)	9	150	120	483	258	28	26	919	4	193	921	287
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	0		0	0		0	275		115	285		230
Storage Lanes	0		0	2		0	1		1	1		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.942			0.985				0.850			0.850
Flt Protected		0.998		0.950			0.950			0.950		
Satd. Flow (prot)	0	1629	0	3225	1662	0	1614	2969	1488	1630	3023	1473
Flt Permitted		0.987		0.950			0.950			0.950		
Satd. Flow (perm)	0	1611	0	3225	1662	0	1614	2969	1488	1630	3023	1473
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		32			6				100			264
Link Speed (mph)		55			35			55			55	
Link Distance (ft)		1272			1716			3436			1917	
Travel Time (s)		15.8			33.4			42.6			23.8	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	0%	1%	1%	0%	4%	1%	3%	12%	0%	2%	10%	1%
Adj. Flow (vph)	9	156	125	503	269	29	27	957	4	201	959	299
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	290	0	503	298	0	27	957	4	201	959	299
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		24			24			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2	1	1	2	1
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right	Left	Thru	Right
Leading Detector (ft)	20	100		20	100		20	100	20	20	100	20
Trailing Detector (ft)	0	0		0	0		0	0	0	0	0	0
Detector 1 Position(ft)	0	0		0	0		0	0	0	0	0	0
Detector 1 Size(ft)	20	6		20	6		20	6	20	20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Prot	NA		Prot	NA	Perm	Prot	NA	Perm
Protected Phases		4		3	8		5	2		1	6	

Lanes, Volumes, Timings  
3: HWY 99 & Barlow Rd

02/06/2019

												
Lane Group	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NET	NER	SWL	SWT	SWR
Permitted Phases	4								2			6
Detector Phase	4	4		3	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		13.0	5.0		13.0	5.0	5.0	8.0	5.0	5.0
Minimum Split (s)	37.5	37.5		17.5	23.0		18.0	23.0	23.0	13.0	23.0	23.0
Total Split (s)	37.5	37.5		22.8	60.3		18.0	43.7	43.7	16.0	41.7	41.7
Total Split (%)	31.3%	31.3%		19.0%	50.3%		15.0%	36.4%	36.4%	13.3%	34.8%	34.8%
Maximum Green (s)	33.0	33.0		18.3	55.8		13.0	39.2	39.2	11.0	37.2	37.2
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	0.5	0.5		0.5	0.5		1.0	0.5	0.5	1.0	0.5	0.5
Lost Time Adjust (s)		-0.5		-0.5	-0.5		-0.5	-0.5	-0.5	-0.5	-0.5	-0.5
Total Lost Time (s)		4.0		4.0	4.0		4.5	4.0	4.0	4.5	4.0	4.0
Lead/Lag	Lag	Lag		Lead			Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None		None	None		None	Max	Max	None	Max	Max
Walk Time (s)	7.0	7.0						7.0	7.0		7.0	7.0
Flash Dont Walk (s)	26.0	26.0						11.0	11.0		11.0	11.0
Pedestrian Calls (#/hr)	0	0						0	0		0	0
Act Effect Green (s)		23.4		18.9	46.3		13.5	39.8	39.8	11.5	45.5	45.5
Actuated g/C Ratio		0.21		0.17	0.42		0.12	0.36	0.36	0.10	0.41	0.41
v/c Ratio		0.79		0.91	0.43		0.14	0.89	0.01	1.18	0.77	0.39
Control Delay		52.1		67.9	23.8		47.7	45.8	0.0	170.1	36.3	7.1
Queue Delay		0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay		52.1		67.9	23.8		47.7	45.8	0.0	170.1	36.3	7.1
LOS		D		E	C		D	D	A	F	D	A
Approach Delay		52.1			51.5			45.7			48.7	
Approach LOS		D			D			D			D	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 110.2

Natural Cycle: 120

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.18

Intersection Signal Delay: 48.8








Intersection LOS: D

Intersection Capacity Utilization 86.6%

ICU Level of Service E

Analysis Period (min) 15






















Splits and Phases: 3: HWY 99 & Barlow Rd

 Ø1	 Ø2	 Ø3	 Ø4
16 s	43.7 s	22.8 s	37.5 s
 Ø5	 Ø6	 Ø8	
18 s	41.7 s	60.3 s	

# HCM Signalized Intersection Capacity Analysis

## 3: HWY 99 & Barlow Rd


















02/06/2019






												
Movement	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Traffic Volume (vph)	9	150	120	483	258	28	26	919	4	193	921	287
Future Volume (vph)	9	150	120	483	258	28	26	919	4	193	921	287
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)		4.0		4.0	4.0		4.5	4.0	4.0	4.5	4.0	4.0
Lane Util. Factor		1.00		0.97	1.00		1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.94		1.00	0.99		1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected		1.00		0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)		1630		3225	1663		1614	2969	1488	1630	3023	1473
Flt Permitted		0.99		0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)		1610		3225	1663		1614	2969	1488	1630	3023	1473
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	9	156	125	503	269	29	27	957	4	201	959	299
RTOR Reduction (vph)	0	25	0	0	4	0	0	0	3	0	0	157
Lane Group Flow (vph)	0	265	0	503	294	0	27	957	1	201	959	142
Heavy Vehicles (%)	0%	1%	1%	0%	4%	1%	3%	12%	0%	2%	10%	1%
Turn Type	Perm	NA		Prot	NA		Prot	NA	Perm	Prot	NA	Perm
Protected Phases		4		3	8		5	2		1	6	
Permitted Phases	4								2			6
Actuated Green, G (s)		22.9		18.4	45.8		7.5	41.5	41.5	11.0	45.0	45.0
Effective Green, g (s)		23.4		18.9	46.3		8.0	42.0	42.0	11.5	45.5	45.5
Actuated g/C Ratio		0.21		0.17	0.41		0.07	0.37	0.37	0.10	0.41	0.41
Clearance Time (s)		4.5		4.5	4.5		5.0	4.5	4.5	5.0	4.5	4.5
Vehicle Extension (s)		3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)		335		542	685		114	1110	556	166	1224	596
v/s Ratio Prot				c0.16	0.18		0.02	c0.32		c0.12	c0.32	
v/s Ratio Perm		c0.16							0.00			0.10
v/c Ratio		0.79		0.93	0.43		0.24	0.86	0.00	1.21	0.78	0.24
Uniform Delay, d1		42.1		46.0	23.6		49.3	32.5	22.0	50.4	29.1	22.0
Progression Factor		1.00		1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2		12.0		22.2	0.4		1.1	8.9	0.0	137.8	5.1	0.9
Delay (s)		54.1		68.2	24.0		50.3	41.3	22.0	188.2	34.2	22.9
Level of Service		D		E	C		D	D	C	F	C	C
Approach Delay (s)		54.1			51.8			41.5			53.1	
Approach LOS		D			D			D			D	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			49.6			HCM 2000 Level of Service				D		
HCM 2000 Volume to Capacity ratio			0.89									
Actuated Cycle Length (s)			112.3			Sum of lost time (s)			16.5			
Intersection Capacity Utilization			86.6%			ICU Level of Service			E			
Analysis Period (min)			15									
c Critical Lane Group												

# Lanes, Volumes, Timings

## 7: Site & Barlow Rd

02/06/2019

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	0	0	6	0	61	0	251	6	47	430	0
Future Volume (vph)	0	0	0	6	0	61	0	251	6	47	430	0
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	0		0	0		0	0		0	150		0
Storage Lanes	0		0	0		0	0		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.878			0.997				
Flt Protected					0.995					0.950		
Satd. Flow (prot)	0	1716	0	0	1499	0	0	1711	0	1630	1716	0
Flt Permitted					0.995					0.950		
Satd. Flow (perm)	0	1716	0	0	1499	0	0	1711	0	1630	1716	0
Link Speed (mph)		30			30			55			55	
Link Distance (ft)		492			522			308			790	
Travel Time (s)		11.2			11.9			3.8			9.8	
Peak Hour Factor	0.85	0.92	0.85	0.92	0.92	0.92	0.89	0.89	0.89	0.90	0.90	0.90
Adj. Flow (vph)	0	0	0	7	0	66	0	282	7	52	478	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	73	0	0	289	0	52	478	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	53.5%						ICU Level of Service A					
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	1.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	0	0	0	6	0	61	0	251	6	47	430	0
Future Vol, veh/h	0	0	0	6	0	61	0	251	6	47	430	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	150	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	85	92	85	92	92	92	89	89	89	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	7	0	66	0	282	7	52	478	0
Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	901	871	478	868	868	286	478	0	0	289	0	0
Stage 1	582	582	-	286	286	-	-	-	-	-	-	-
Stage 2	319	289	-	582	582	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	259	289	587	273	290	753	1084	-	-	1273	-	-
Stage 1	499	499	-	721	675	-	-	-	-	-	-	-
Stage 2	693	673	-	499	499	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	229	277	587	265	278	753	1084	-	-	1273	-	-
Mov Cap-2 Maneuver	229	277	-	265	278	-	-	-	-	-	-	-
Stage 1	499	479	-	721	675	-	-	-	-	-	-	-
Stage 2	632	673	-	479	479	-	-	-	-	-	-	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s	0		11.3		0		0.8					
HCM LOS	A		B									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR					
Capacity (veh/h)	1084	-	-	-	646	1273	-	-				
HCM Lane V/C Ratio	-	-	-	-	0.113	0.041	-	-				
HCM Control Delay (s)	0	-	-	0	11.3	7.9	-	-				
HCM Lane LOS	A	-	-	A	B	A	-	-				
HCM 95th %tile Q(veh)	0	-	-	-	0.4	0.1	-	-				



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